



City of Austin Residential New Construction and Addition Permit Application

Residential Review, 2nd floor, One Texas Center
505 Barton Springs, Austin, TX 78704 (512) 978-4000

Property Information

Project Address: 3701 Westlake Dr.	Tax Parcel ID: 123437 and 123438
Legal Description: N 29.4' of Lot 87 & S 70.6 FRNTG Rd of Lot 88 & 2X8.11 Water Well St Lake Shore Addn.	
Zoning District: LA 3501: LOT 2 BUELL - RUDE SUPP.	Lot Area (sq ft): 59,470
Neighborhood Plan Area (if applicable): N/A	Historic District (if applicable): N/A

Required Reviews

Is project participating in S.M.A.R.T. Housing? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, attach signed certification letter from NHCD, and signed conditional approval letter from Austin Energy Green Building)	Does project have a Green Building requirement? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, attach signed conditional approval letter from Austin Energy Green Building)
Is this site within an Airport Overlay Zone? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, approval through Aviation is required)	Does this site have a septic system? <input checked="" type="radio"/> Y <input type="radio"/> N (If yes, submit a copy of approved septic permit)
Does the structure exceed 3,600 square feet total under roof? <input checked="" type="radio"/> Y <input type="radio"/> N (If yes, Fire review is required)	Is this property within 200 feet of a hazardous pipeline? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, Fire review is required)
Is this site located within an Erosion Hazard Zone? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, EHZ review is required)	Is this property within 150 feet of the 100 year floodplain? <input checked="" type="radio"/> Y <input type="radio"/> N (Proximity to floodplain may require additional review time.)
Is there a protected sized tree on this lot or adjacent lot(s)? <input checked="" type="radio"/> Y <input type="radio"/> N Note: Include tree location(s) on plot plan.	(If yes, application for a tree permit with the City Arborist is required) SEE APPROVED TREE PERMIT
Is this site within the Residential Design and Compatibility Standards Ordinance Boundary Area? (LDC 25-2 Subchapter F) <input type="radio"/> Y <input checked="" type="radio"/> N	
Does this site currently have: water availability? <input checked="" type="radio"/> Y <input type="radio"/> N wastewater availability? <input checked="" type="radio"/> Y <input type="radio"/> N (If no, contact Austin Water Utility to apply for water/wastewater taps and/or service extension request.)	
Are there existing water/wastewater infrastructure, appurtenances or existing water/wastewater easements located on site? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, contact Austin Water Utility Pipeline Engineering for review and approval)	
Does this site have or will it have an auxiliary water source? <input type="radio"/> Y <input checked="" type="radio"/> N (Auxiliary water supplies are wells, rainwater harvesting, river water, lake water, reclaimed water, etc.) (If yes, submit approved auxiliary and potable plumbing plans.)	
Does this site require a cut or fill in excess of four (4) feet? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, contact the Development Assistance Center for a Site Plan Exemption)	
Is this site within the Waterfront Overlay? <input type="radio"/> Y <input checked="" type="radio"/> N (LDC 25-2 Subchapter C Article 3)	Is this site within the Lake Austin Overlay? <input checked="" type="radio"/> Y <input type="radio"/> N (LDC 25-2-180, 25-2-647)
Does this site front a paved street? <input checked="" type="radio"/> Y <input type="radio"/> N (If no, contact Development Assistance Center for Site Plan requirements.)	Is this site adjacent to a paved alley? <input type="radio"/> Y <input checked="" type="radio"/> N (Public Works approval required to take access from a public alley.)
Does this site have a Board of Adjustment (BOA) variance? <input type="radio"/> Y <input checked="" type="radio"/> N Case # _____ (if applicable)	
Does this site have a Residential Design and Compatibility Commission (RDCC) waiver? <input type="radio"/> Y <input checked="" type="radio"/> N (If yes, provide a copy of decision sheet. Note: A permit cannot be approved within 10 days of approval of a variance from BOA.)	

Description of Work

Existing Use: <input checked="" type="radio"/> vacant <input type="radio"/> single-family residential <input type="radio"/> duplex residential <input type="radio"/> two-family residential <input type="radio"/> other: _____			
Proposed Use: <input type="radio"/> vacant <input checked="" type="radio"/> single-family residential <input type="radio"/> duplex residential <input type="radio"/> two-family residential <input type="radio"/> other: _____			
Project Type: <input checked="" type="radio"/> new construction <input type="radio"/> addition <input type="radio"/> addition/remodel <input type="radio"/> other: _____			
Will all or part of an existing exterior wall, structure, or roof be removed as part of the project? <input type="radio"/> Y <input checked="" type="radio"/> N (Note: Removal of all or part of a structure requires a demolition permit application.)			
# of existing bedrooms: 0	# of bedrooms upon completion: 7	# of baths existing: 0	# of baths upon completion: 13

Project Description: (Note: Please provide thorough description of project. Attach additional pages as necessary.)
Please see separate page for project description.

Trades Permits Required (Circle as applicable): electric plumbing mechanical (HVAC) concrete (R.O.W.)

Job Valuation						
Total Job Valuation: \$ <u>19 MILLION</u> <small>Note: The total job valuation should be the sum total of all valuations noted to the right. Labor and materials only, rounded to nearest dollar. Permit fees are based on adopted fee schedule.</small>	Amount of Total Job Valuation dedicated to all Addition and/or New Construction: \$ <u>19 MILLION</u> Amount for Primary Structure: \$ _____ Elec: <input checked="" type="checkbox"/> Y <input type="checkbox"/> N Plmbg: <input checked="" type="checkbox"/> Y <input type="checkbox"/> N Mech: <input type="checkbox"/> N <input type="checkbox"/> N Amount for Accessory Structure: \$ _____ Elec: <input type="checkbox"/> Y <input type="checkbox"/> N Plmbg: <input type="checkbox"/> Y <input type="checkbox"/> N Mech: <input type="checkbox"/> Y <input type="checkbox"/> N			Amount of Total Job Valuation dedicated to all Remodel/Repair: Bldg: \$ _____ Elec: \$ _____ Plmbg: \$ _____ Mech: \$ _____ TOTAL: \$ <u>0</u>		
Please utilize the Calculation Aid on the last page of the Additional Information, page 7, as a guide to complete the following calculations and to provide supplemental information for thorough review.						
Site Development Information						
Area Description <small>Note: Provide a separate calculation for each distinct area. Attach additional sheets as necessary. Measurements are to the outside surface of the exterior wall.</small>	Existing Sq Ft		New/Added Sq Ft		Total Sq Ft	
	Bldg 1	Bldg 2	Bldg 1	Bldg 2	Bldg 1	Bldg 2
a) 1 st Floor conditioned area			7926		<u>7926</u>	0.00
b) 2 nd Floor conditioned area			7433		<u>7433</u>	0.00
c) 3 rd Floor conditioned area			6161		<u>6161</u>	0.00
d) Basement			7120		<u>7120</u>	0.00
e) Covered parking (garage or carport)					0.00	0.00
f) Covered patio, deck, porch, and/or balcony area(s)			3659		<u>3659</u>	0.00
g) Other covered or roofed area			359		<u>359</u>	0.00
h) Uncovered wood decks					0.00	0.00
Total Building Area (total a through h)	0.00	0.00	0.00	0.00	0.00	0.00
i) Pool			1447		<u>1447</u>	0.00
j) Spa					0.00	0.00
Building Coverage Information						
<small>Note: Building Coverage means the area of a lot covered by buildings or roofed areas, but excludes ground-level paving, landscaping, open recreational facilities, incidental projecting eaves, balconies, and similar features. Pools, ponds, and fountains are not included in this measurement. (LDC 25-1-21)</small>						
Total Building Coverage (sq ft): <u>9571</u> % of lot size: <u>16.09%</u>						
Impervious Cover Information						
<small>Note: Impervious cover is the total horizontal area of covered spaces, paved areas, walkways, and driveways. The term excludes pools, ponds, fountains, and areas with gravel placed over pervious surfaces that are used only for landscaping or by pedestrians. For an uncovered wood deck that has drainage spaces between the deck boards and that is located over a pervious surface, 50 percent of the horizontal area of the deck is included in the measurement of impervious cover. (LDC 25-1-23)</small>						
Total Impervious Cover (sq ft): <u>16236</u> % of lot size: <u>27.30%</u>						
Setbacks						
Are any existing structures on this site a non-compliant structure based on a yard setback requirement? (LDC 25-2-492) <input type="radio"/> Y <input checked="" type="radio"/> N						
Does any structure (or an element of a structure) extend over or beyond a required yard? (LDC 25-2-513) <input type="radio"/> Y <input checked="" type="radio"/> N						
Is front yard setback averaging being utilized on this property? (LDC 25-2, Subchapter F, Sec. 2.3 or 25-2-778) <input type="radio"/> Y <input checked="" type="radio"/> N						
Height Information (LDC 25-1-21 or 25-2 Subchapter F, Section 3.4)			Parking (LDC 25-6 Appendix A & 25-6-478)			
Building Height: <u>34</u> ft <u>6</u> in Number of Floors: <u>4</u>			# of spaces required: <u>2</u> # of spaces provided: <u>2</u>			
Right-of-Way Information						
Is a sidewalk required for the proposed construction? (LDC 25-6-353) <input checked="" type="radio"/> Y <input type="radio"/> N <small>*Sidewalks are to be installed on any new construction of a single family, two-family or duplex residential structure and any addition to an existing building that increases the building's gross floor area by 50 % or more.</small>						
Will a Type I driveway approach be installed, relocated, removed or repaired as part of this project? <input checked="" type="radio"/> Y <input type="radio"/> N						
Width of approach (measured at property line): <u>22.5</u> ft Distance from intersection (for corner lots only): <u>N/A</u> ft						
Are storm sewer inlets located along the property or within ten (10) feet of the boundaries of the property? (If yes, drainage review is required) <input type="radio"/> Y <input checked="" type="radio"/> N						

Subchapter F

Gross Floor Area

This section is only required for projects located within the Residential Design and Compatibility Standards Ordinance Boundaries as defined and illustrated in Title 25-2 Subchapter F of the Land Development Code. The Gross Floor Area of each floor is measured as the area contained within the outside edge of the exterior walls.

	Existing Sq Ft	New/Added Sq Ft	Proposed Exemption (check article utilized)	Applied Exemption Sq Ft	Total Sq Ft
1 st Floor					0.00
2 nd Floor					0.00
3 rd Floor					0.00
Area w/ ceilings > 15'			Must follow article 3.3.5		0.00
Ground Floor Porch* (check article utilized)			<input checked="" type="checkbox"/> Full Porch sq ft (3.3.3 A) <input checked="" type="checkbox"/> 200 sq ft (3.3.3 A 2)		0.00
Basement			Must follow article 3.3.3B, see note below		0.00
Attic			Must follow article 3.3.3C, see note below		0.00
Garage**: (check article utilized)	Attached		<input type="checkbox"/> 200 sq ft (3.3.2 B 2b)		0.00
	Detached		<input checked="" type="checkbox"/> 450 sq ft (3.3.2 A 1 / 2a) <input checked="" type="checkbox"/> 200 sq ft (3.3.2 B 2a)		0.00
Carport**: (check article utilized)	Attached		<input checked="" type="checkbox"/> 450 sq ft (3.3.2 A 3) <input checked="" type="checkbox"/> 200 sq ft (3.3.2 B 1)***		0.00
	Detached		<input type="checkbox"/> 450 sq ft (3.3.2 A 1)		0.00
Accessory Building(s) (detached)					0.00
Totals	0.00	0.00			0.00

TOTAL GROSS FLOOR AREA (add Total Sq Ft column) 0.00

(Total Gross Floor Area ÷ Lot Area) x 100 = _____ Floor-To-Area Ratio (FAR)

Is a sidewall articulation required for this project? ☐ Y ☐ N

(Yes, if: a wall, 15' tall or higher, within 9 feet of a side property line extends further than 36 feet in length per article 2.7.1)

Does any portion of the structure extend beyond a setback plane/exemption exhibit (aka "tent")? ☐ Y ☐ N

(If Yes, indicate applicable section of Subchapter F and length of protrusion on the drawings.)

***Ground Floor Porch exemption:** A ground floor porch, including a screened porch, may be exempted, provided that the porch is not accessible by automobile and is not connected to a driveway; and the exemption may not exceed 200 square feet if a porch has habitable space or a balcony above it.

****Garage and carport exemptions (in relation to primary structure):** Exemptions must follow the code as outlined in Title 25-2 Subchapter F 3.3.2. Each amount listed (450 or 200) is the maximum exclusion allowed per the article designated. Note: Article 3.3.2 C, "An applicant may receive only one 450-square foot exemption per site under paragraph A. An applicant who receives a 450-square foot exemption may receive an additional 200-foot exemption for the same site under paragraph B, but only for an attached parking area used to meet minimum parking requirements."

*****Ordinance article 3.3.2 B 1** is the only 200 sq ft exemption that may be combined with a 450 sq ft exemption. Otherwise only one 450 exemption or one 200 sq ft exemption may be taken.

Basement exemption: A habitable portion of a building that is below grade may be exempted if the habitable portion does not extend beyond the first-story footprint and is below natural or finished grade, whichever is lower; and it is surrounded by natural grade for at least 50% of its perimeter wall area and the finished floor of the first story is not more than three feet above the average elevation at the intersections of the minimum front yard setback line and the side property lines.

Habitable Attic exemption: A habitable portion of an attic may be exempted if: 1) The roof above it is not a flat or mansard roof and has a slope of 3 to 12 or greater; 2) It is fully contained within the roof structure; 3) It has only one floor; 4) It does not extend beyond the footprint of the floors below; 5) It is the highest habitable portion of the building, or a section of the building, and adds no additional mass to the structure; and 6) Fifty percent or more of the area has a ceiling height of seven feet or less.

Contact Information			
Owner	Bryan & Sharoll Sheffield	Applicant/Agent	Mindy Briggs, Permit Partners
Mailing Address	PO Box 1563, Austin 78746	Mailing Address	105 W. Riverside Dr. 78704
Phone		Phone	512-593-5361
Email		Email	mindy@permit-partners.com
General Contractor	DALGLEISH CONSTRUCTION	Design Professional	Rhotenberry Wellen Architects
Mailing Address	4019 Spice Wood Springs Rd	Mailing Address	1102 W. Texas Ave, Midland, 79701
Phone	92-346-8554 78701	Phone	(432) 682-1252
Email	NICK@DALGLEISH.NET	Email	cale@rwarchitects.com

Authorization

22 I understand that in accordance with Sections 25-1-111 and 25-11-66 of the Land Development Code (LDC), non-compliance with the LDC may be cause for the Building Official to suspend or revoke a permit and/or license.

22 I acknowledge that this project qualifies for the Site Plan Exemption as listed in Section 25-5-2 of the LDC. I understand that nothing may be built upon or over an easement.

22 I further understand that no portion of any roof structure may overhang in any public utility or drainage easement. I acknowledge that customer will bear the expense of any necessary relocation of existing utilities to clear this driveway location and/or the cost to repair any damage to existing utilities caused during construction. Water services, meters, and wastewater cleanouts are not permitted within or beneath driveways or sidewalks. Private plumbing appurtenances will not be located in public right-of-way or public easements. Private plumbing lines will not cross lot lines.

22 I agree that this application will expire on the 181st day after the date that the application is filed if the application is not approved and an extension is not granted. If the application expires, a new submittal will be required and compliance with current code may be required.

22 I hereby certify that to the best of my knowledge and ability, the information provided in this application is complete and accurate.

22 I further acknowledge that, should any information contained herein prove incorrect, the building official may suspend or revoke any resulting permit and/or license.

22 I also understand that if there are any trees greater than 19 inches in diameter located on the property and immediately adjacent to the site, I am required to complete a Tree Ordinance Review Application by contacting (512) 974-1876 or cityarborist@austintexas.gov. This initiates the tree permitting requirement needed to proceed with the development review process.

22 I also understand that if there is a septic system located on the property, I am required to complete an On-site Sewage Facility (a.k.a. an OSSF or septic system) application by contacting Austin Water at (512) 972-4050 or ossf@austintexas.gov. This initiates the septic system permitting requirement needed to proceed with the development review process.

22 Erosion and Sedimentation Controls are required per Section 25-8-181 of the LDC. Failure to comply with this requirement may result in a Stop Work Order and/or legal action by the City of Austin including criminal charges and fines of up to \$2,000.00 per day.

22 I acknowledge that a sidewalk will be required on any new construction of a single family, two-family or duplex residential structure and any addition to an existing building that increases the building's gross floor area by 50 % or more.

22 I acknowledge if my plans are subject to a technical review it will not be construed to be a permit for, or an approval of any violation of any of the provisions of the current adopted building codes or another ordinance of the City of Austin.

22 I am the record owner of this property and authorize the agent/applicant listed above to apply for and acquire a permit on my behalf.

Applicant's signature: [Signature] Date: 4/25/15

Owner's signature: [Signature] Date: 4-25-16

Design Professional's signature: [Signature] Date: 4.25.16

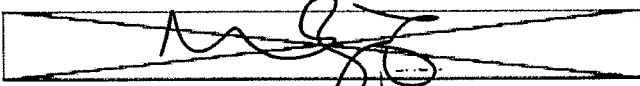
General Contractor's signature: _____ Date: _____

One Stop Shop
505 Barton Springs Rd
(512) 974-2632 – phone
(512) 974-9112 – phone
(512) 974-9109 – fax
(512) 974-9779 – fax

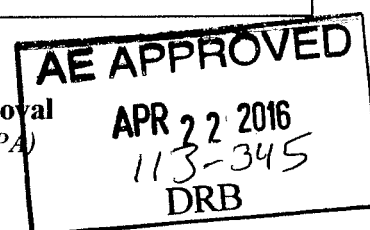


Austin Energy
Building Service Planning Application (BSPA)

This form to be used for review of Building Permit only

Responsible Person for Service Request <u>Mindy Briggs</u>		
Email <u>mindy@permit-partners.com</u>	Fax _____	Phone <u>512-593-5361</u>
<input checked="" type="checkbox"/> Residential	<input type="checkbox"/> Commercial	<input checked="" type="checkbox"/> New Construction
<input type="checkbox"/> Remodeling		
Project Address <u>3701 Westlake Dr.</u> OR		
Legal Description _____	Lot _____	Block _____
Who is your electrical provider? <input checked="" type="checkbox"/> AE <input type="checkbox"/> Other _____		
<input checked="" type="checkbox"/> Overhead Service	<input type="checkbox"/> Underground Service	<input checked="" type="checkbox"/> Single-phase (1Ø) <input type="checkbox"/> Three-phase (3Ø)
Location of meter _____		
Number of existing meters on gutter _____ (show all existing meters on riser diagram)		
Expired permit # _____		
Comments <u>Construction of new 2-story 32,658sf single-family residence with basement level and pool.</u>		
	Date <u>4/22/16</u>	Phone <u>512-593-5361</u>
ESPA Completed by (Signature & Print Name) _____		
Approved <input type="checkbox"/> Yes <input type="checkbox"/> No _____		
AE Representative _____	Date _____	Phone _____

All structures must maintain
7'6" clearance over energized
distribution power lines. Enforced
by AE and NESC codes-this review
DOES NOT include transmission
power lines.





Water & Wastewater Service Plan Verification (WWWSPV)

Service Address: 3701 Westlake Dr.

Lot: _____ Block: _____ Subdivision: _____

Existing Use: vacant

Proposed Use: Single-Family Res. Two-Family Res. Duplex Mobile Home Secondary Apt. Cottage Urban Home

Guest House Accessory Dwelling Accessory Apt. Other _____

Existing # Baths 0 Additional # Baths 13 ^{Text} Total number of bathrooms the meter will serve 13

Mindy Briggs, Permit Partners
Applicant's Name & Title

4/25/16
Date

512-593-5361
Phone

City of Austin Office Use

Water main size _____ Service stub size _____ Service stub upgrade required: Y N

Shared Dual Service? Y N If yes, state meter size(s) on shared service _____

Existing meter # _____ Existing meter size _____

Existing water service line/meter location _____

Upgrade required: Y N New meter(s) size _____

WW main size _____ WW Service line/clean-out location _____

Secondary address needed at property: Y N

AWU Pipeline Engineering approval required: Y N

Utility Tap Plan required: Y N

Comments: Not City of Austin water/ww -
No SPV needed

REVIEWED

APR 25 2016

AUSTIN WATER UTILITY
CONSUMER SERVICE DIVISION - TAPS

AWU Engineer Representative

AWU Taps Representative

Date

4/25/16
Date

Phone

Phone

Water meters & wastewater clean-outs are not permitted in sidewalks or driveways.

Relocation of services to remove them from proposed sidewalks or driveways shall be performed at the applicant's expense.

If the existing water meter was pulled for non-billing account during demolition contact Customer Care at 512-494-9400 to have account set up and the same size meter reinstalled within 120 days of meter removal (with active building permit) to avoid city re-connect charges being applied.



April 25, 2016

Re: 3701 Westlake Drive
Austin, TX 78746
Project Description

1102 West Texas Avenue
Midland, Texas 79701
432.682.1252
432.682.1257 fax
RWArchitects.com

- The project located at 3701 Westlake Dr. will be a single family residence located within the Lake Austin Overlay District.
- All structures to be built will comply with all applicable City of Austin residential building codes and the Lake Austin zoning district.
- The scope of work consists of a 28,640sf, single-family residence with a Basement, First Floor, Second Floor, and Third Floor and three, 2-car garages. Also to be constructed is 308sf uninhabitable Auxiliary Building which will contain building support systems.
- There will also be a 1447sf in-ground pool as well as a 330sf reflecting pool at the front of the property.
- A boat dock will also be constructed along the shoreline of Lake Austin. This structure will be submitted for permit and constructed separately from the single-family residence and at a later date still to be determined.

Sincerely,

Mark T. Wellen, FAIA

MTW:bv

2015-074404 TP



Tree Ordinance Review Application

Planning and Development Review Department

One Texas Center, 505 Barton Springs Road, 4th floor, Austin, TX 78704

Phone: (512) 974-1876 Fax: (512) 974-3010

Email: cityarborist@austintexas.gov Website: www.austintexas.gov/departments/city-arborist

ROW I.D. 11443840
Mapsco Pg. 554E

Application request* (specify all that apply):

- ☐ Tree removal (LDC 25-8-602[3])
☒ Critical Root Zone impacts (ECM 3.5.2 A)
☐ Live canopy impacts of more than 25% (ECM 3.5.2 B)

* Refer to Land Development Code (LDC) 25-8 (B)(1) and Environmental Criteria Manual (ECM) (Section 3 & App. F). Applicant understands that all impacts may threaten the health of the tree and that approval of this application does not guarantee favorable tree results.

Address and zip code of property: 3701 WESTLAKE DR. AUSTIN TX 78746

Name of owner or authorized agent: BARTLETT TREE EXPERTS NICHOLAS CROWTHER

Building permit number (if applicable): _____

Telephone #: 512.450.2332 Fax #: 512.310.8074 E-mail: _____

Tree Species: LIVE OAK (6) LIVE OAK (30) Tree location on lot: REFER TO ATTACHED SITE MAP

Trunk size (in inches) at 4 1/2 feet above ground: circumference (around) _____ or diameter (across) (40), (29.5)

General tree condition: ☒ Good / ☐ Fair / ☐ Poor / ☐ Dead

Reason for request: ☒ Development ☐ Tree condition ☐ Other: INSTALLATION OF SEPTIC

POOD.

Nicholas Crowther

11/10/2015 ✓

Owner/ Authorized Agent Signature

Date

- o Proposed development projects are to include a plan view drawing that depicts the location of the tree and the planned improvements (e.g. structure, driveway, utility and irrigation lines).
- o This permit application only reviews for compliance with tree regulations.
- o The application fee must be paid prior to permit issuance. No fee is required for dead or diseased trees.

Application Determination – To be completed by City Arborist Program Personnel

- ☐ Approved ☒ ^{KA} Approved With Conditions ☐ Statutory Denial (more information required) ☐ Denied

Comments: Build per plans

- ☒ Heritage Tree(s) ☐ A heritage tree variance is required: ☐ Administrative / ☐ Land Use Commission

Conditions of Approval: ☐ None or ☒ As described within Arborist Comments (see above); and

- ☐ Applicant agrees to plant _____ caliper inches of central Texas native trees (see ECM Appendix F) on the lot prior to obtaining a final inspection (if applicable). Trees are to have a minimum 2-inch trunk diameter. Examples include Oaks, Cedar Elm, Bald Cypress, Desert Willow, Mountain Laurel, Texas Persimmon, Mexican Plum, etc.
- ☒ Prior to development, applicant agrees to supply a root zone mulch layer and maintain tree protection fencing (chain-link, five-foot in height) throughout the project duration.
- ☒ No additional impacts are permitted within the 1/2 Critical Root Zone, including utility trenching.
- ☐ Provide a receipt from a certified arborist for: ☐ remedial root care ☐ any required pruning

Applicant Signature

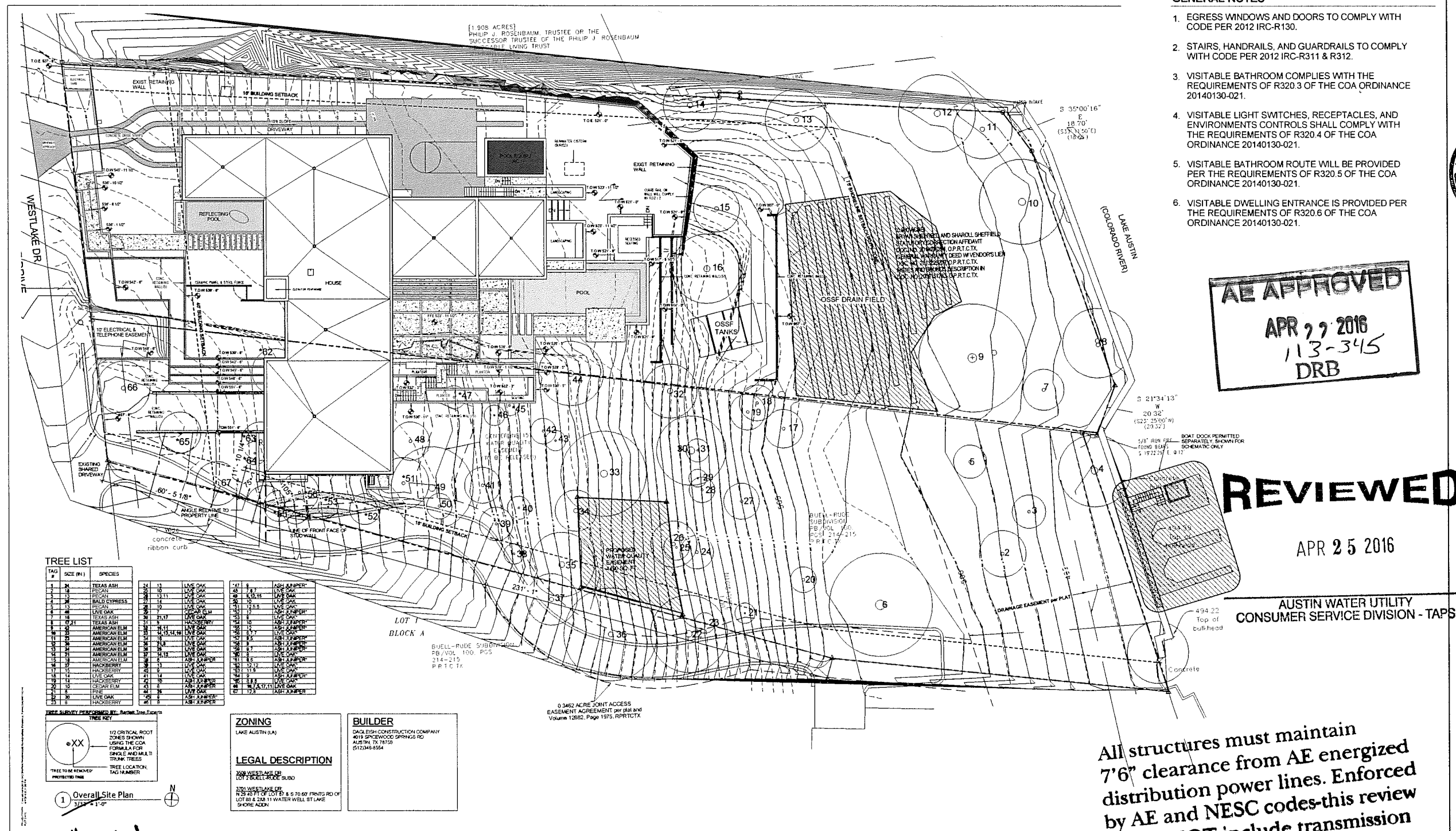
Date

City Arborist Signature

Date

Post this document on site while any proposed work is in progress.
Conditions for approval of this application must be met within 1 year of the effective date.

v 7/2012



GENERAL NOTES

- EGRESS WINDOWS AND DOORS TO COMPLY WITH CODE PER 2012 IRC-R130.
- STAIRS, HANDRAILS, AND GUARDRAILS TO COMPLY WITH CODE PER 2012 IRC-R311 & R312.
- VISITABLE BATHROOM COMPLIES WITH THE REQUIREMENTS OF R320.3 OF THE COA ORDINANCE 20140130-021.
- VISITABLE LIGHT SWITCHES, RECEPTACLES, AND ENVIRONMENTS CONTROLS SHALL COMPLY WITH THE REQUIREMENTS OF R320.4 OF THE COA ORDINANCE 20140130-021.
- VISITABLE BATHROOM ROUTE WILL BE PROVIDED PER THE REQUIREMENTS OF R320.5 OF THE COA ORDINANCE 20140130-021.
- VISITABLE DWELLING ENTRANCE IS PROVIDED PER THE REQUIREMENTS OF R320.6 OF THE COA ORDINANCE 20140130-021.

AE APPROVED
APR 22 2016
113-345
DRB

REVIEWED
APR 25 2016

AUSTIN WATER UTILITY
CONSUMER SERVICE DIVISION - TAPS

All structures must maintain
7'6" clearance from AE energized
distribution power lines. Enforced
by AE and NESC codes-this review
DOES NOT include transmission
power lines.

1102 West Texas Avenue
Midland, Texas 79701
432.682.1252
432.682.1257 fax

rwa
rhotenberry
wellen
architects



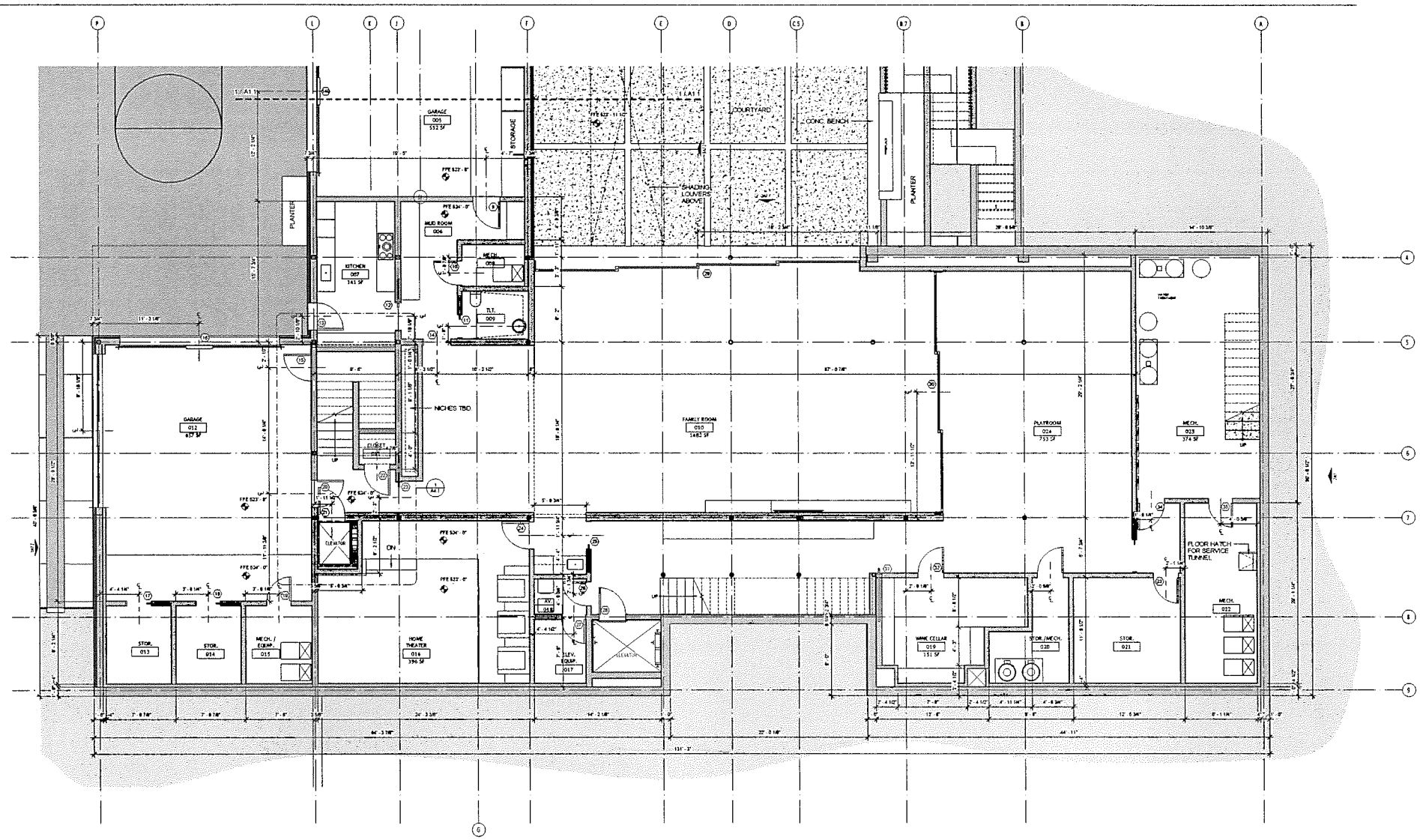
04/08/16

NOTE: Sheet is formatted to
30"x48". Scales are 25% of
noted when printed on
11x17 paper.

Westlake Residence
3701 Westlake Drive
Austin, TX

JOB NO.	1342
DRAWING TITLE	
Basement	
REVISIONS	
DATE	04/07/16
DRAWN BY	NCL
CHECKED BY	MTW
SHEET NO.	

A0.1



1 BASEMENT
1/4" = 1'-0"
1/16" = 1'

GENERAL NOTES

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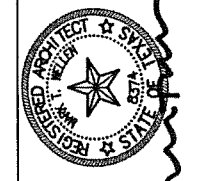
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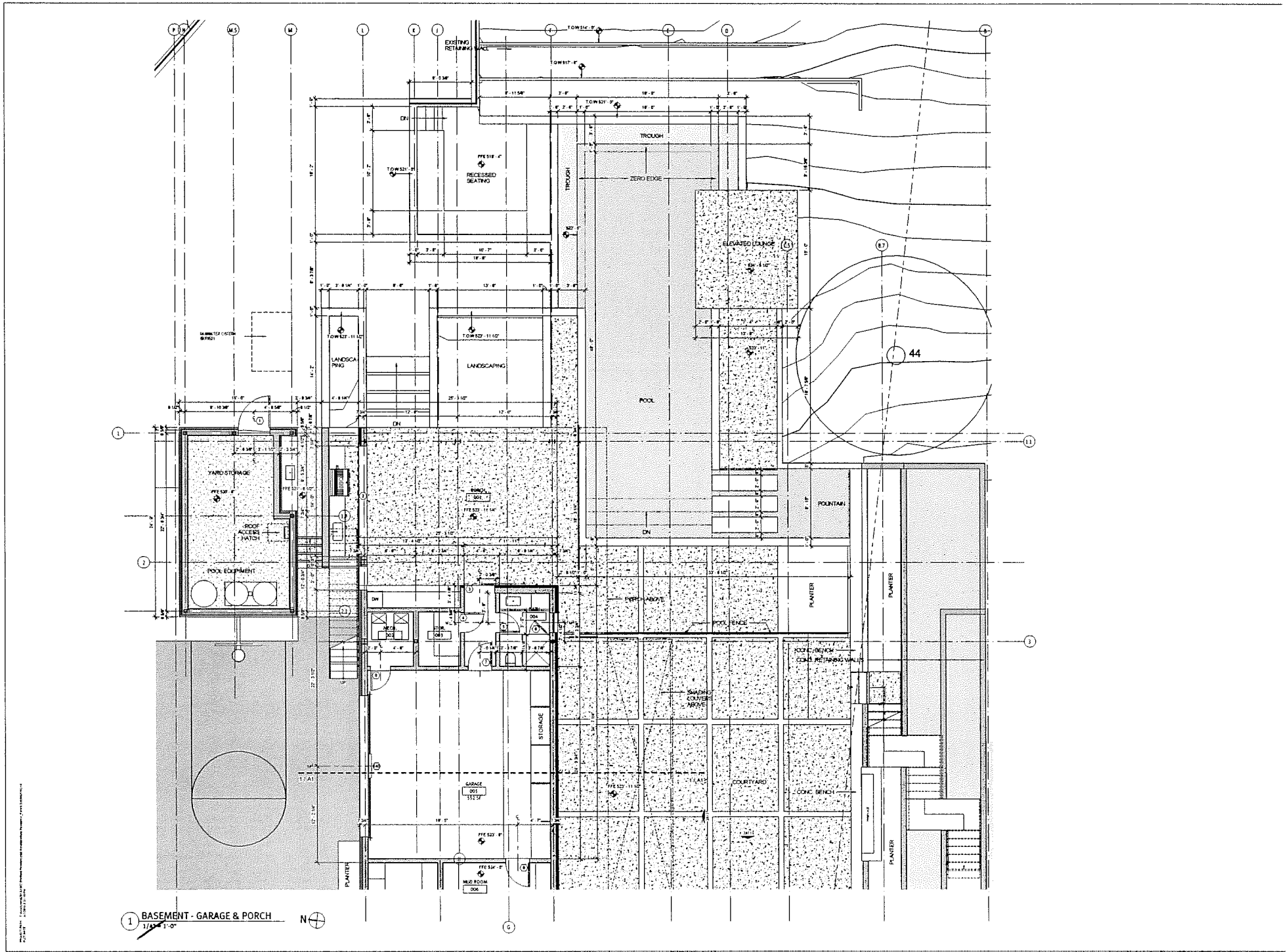
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1/16" = 1'

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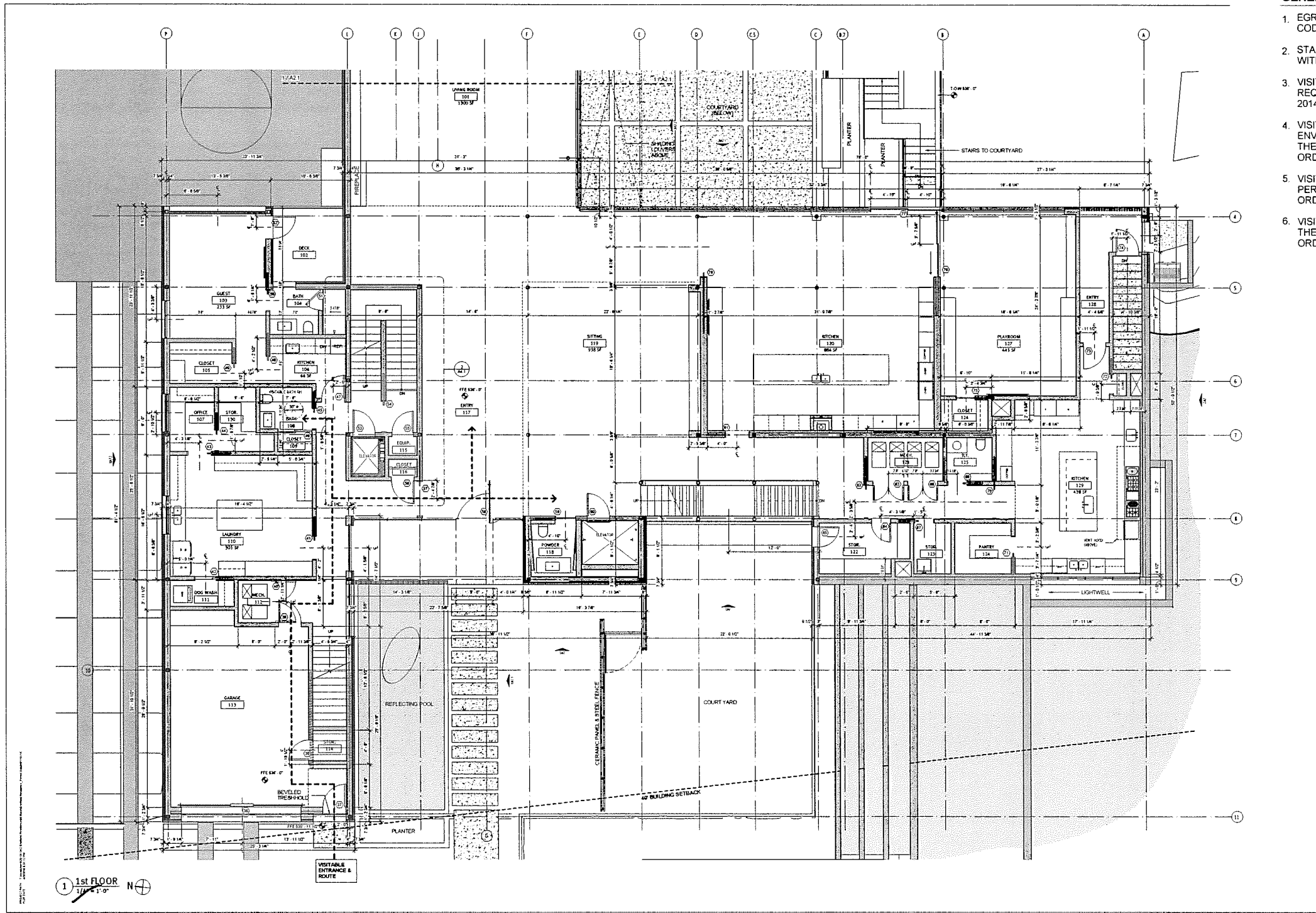


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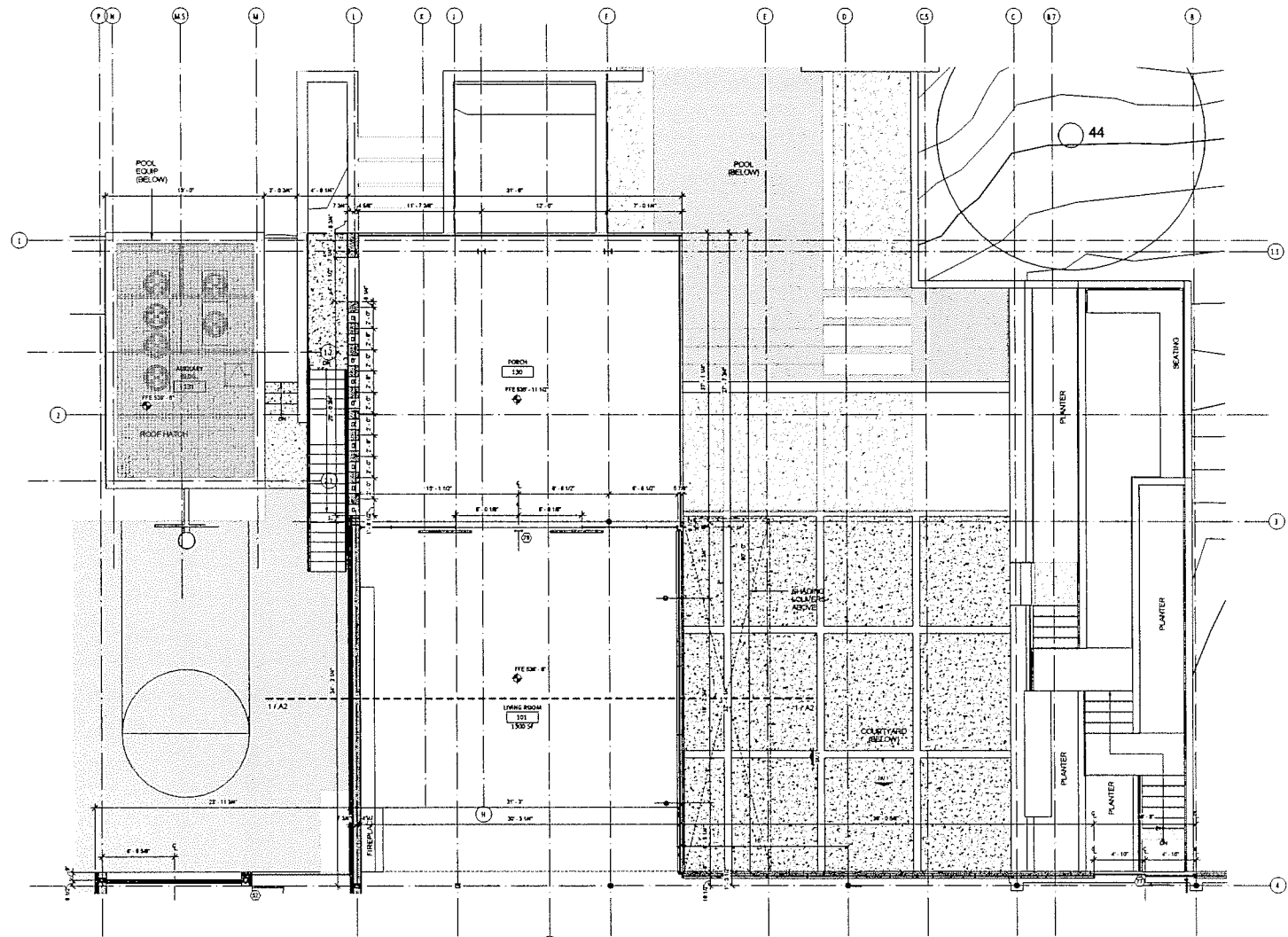
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1 1st FLOOR - LIVING ROOM & PORCH
1/16" = 1'

1/16" = 1'

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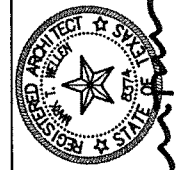
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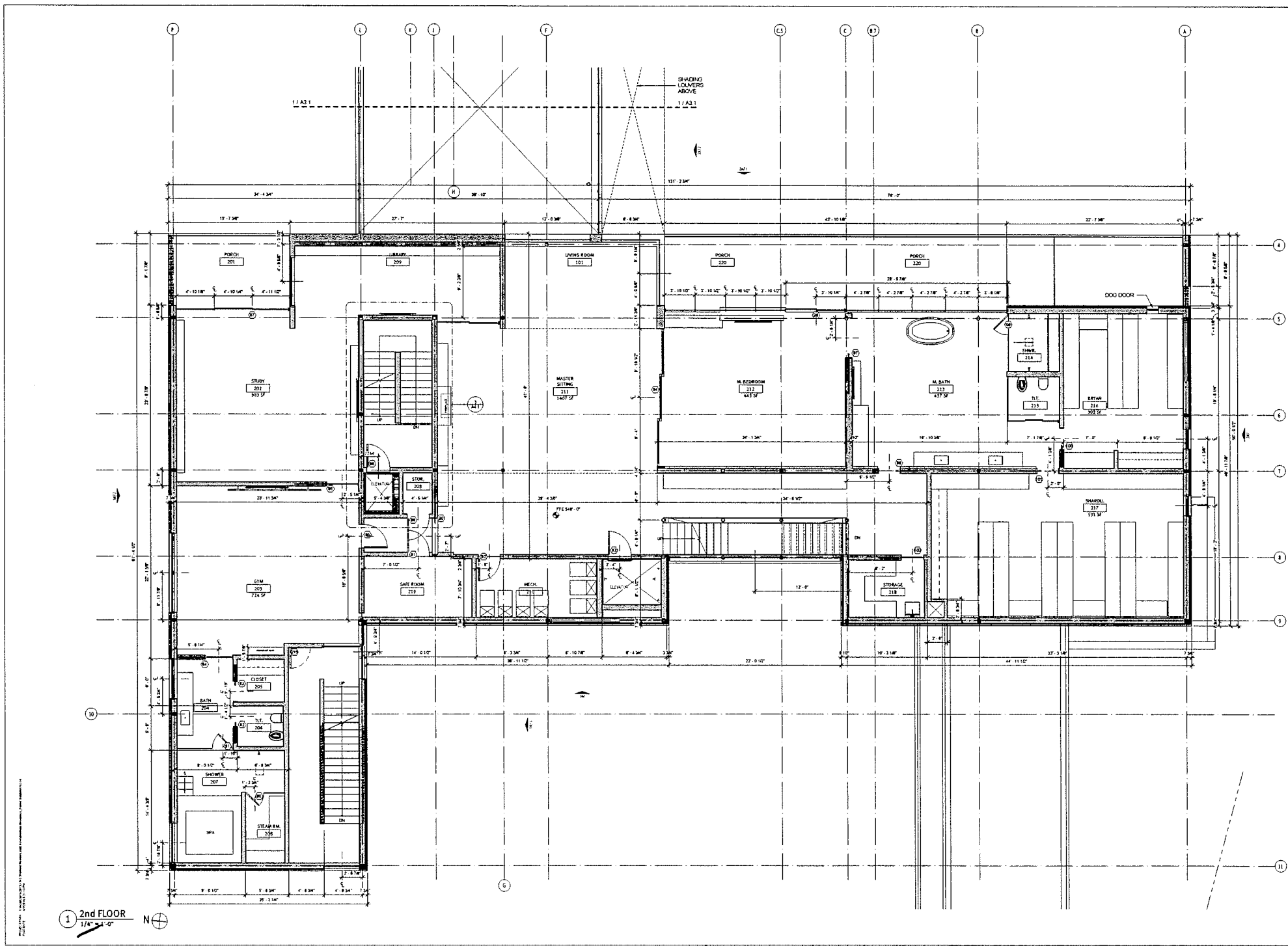
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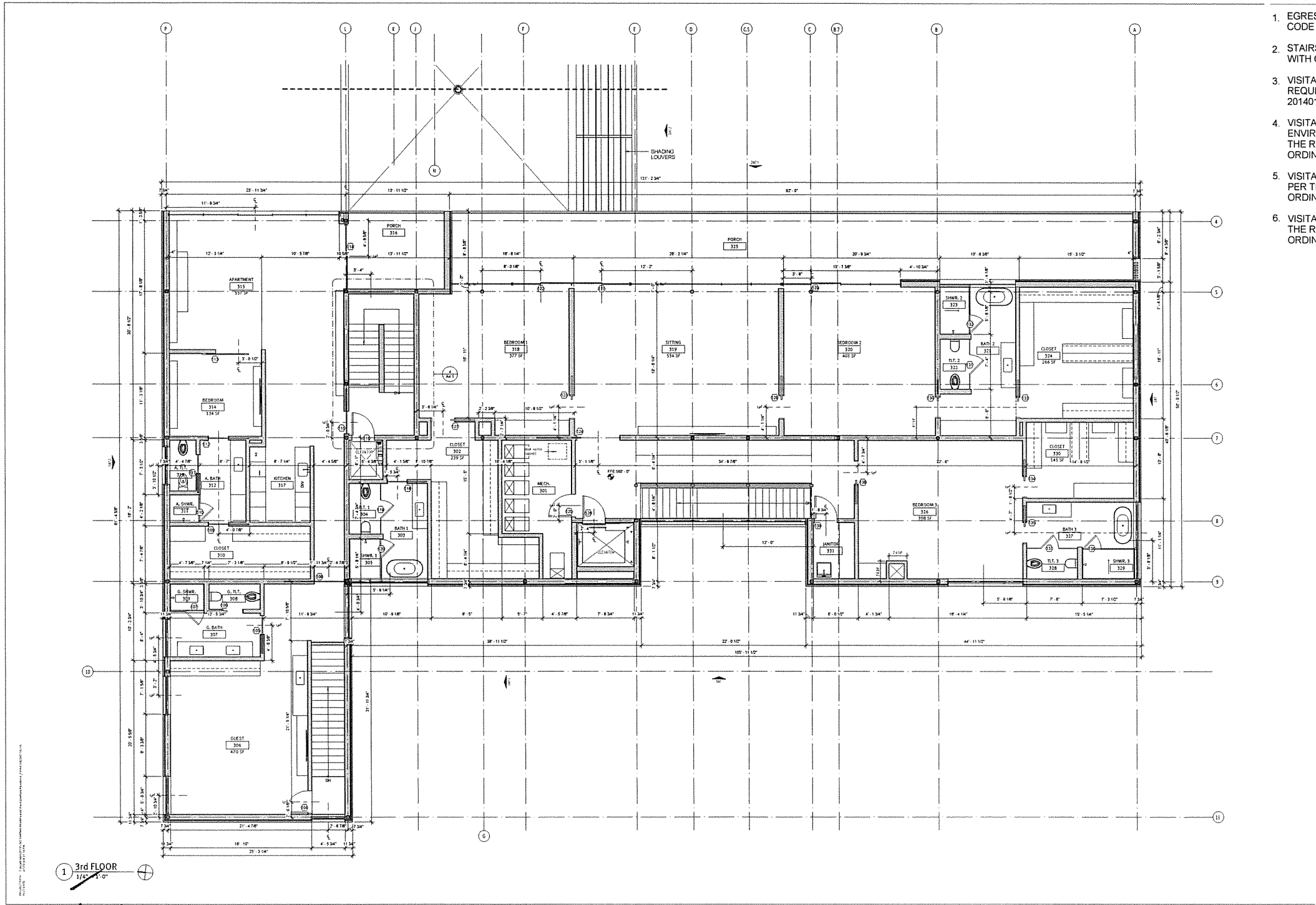
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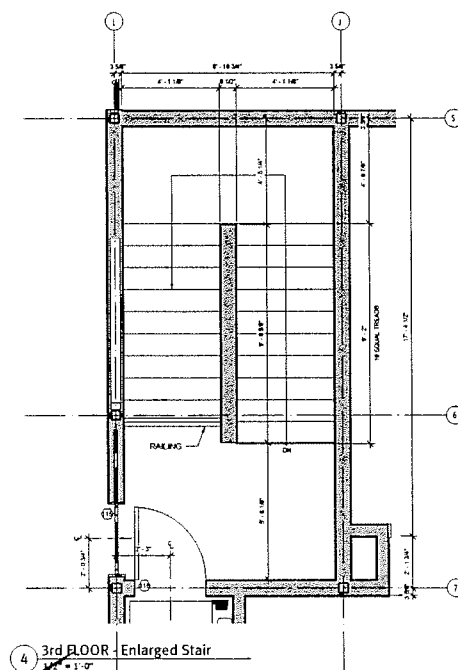
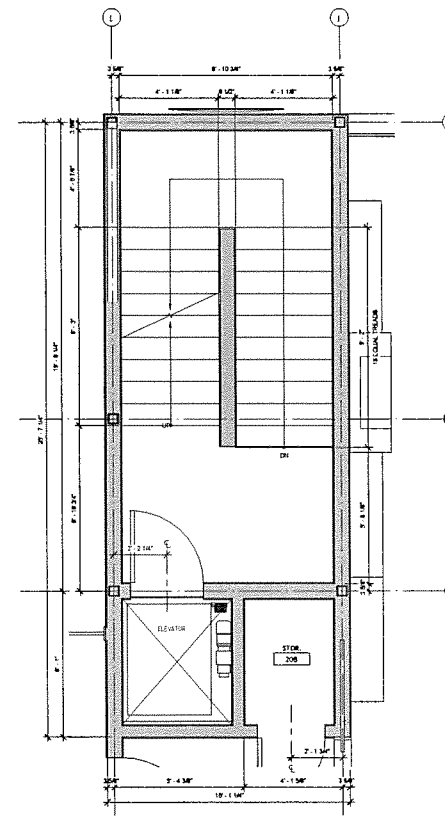
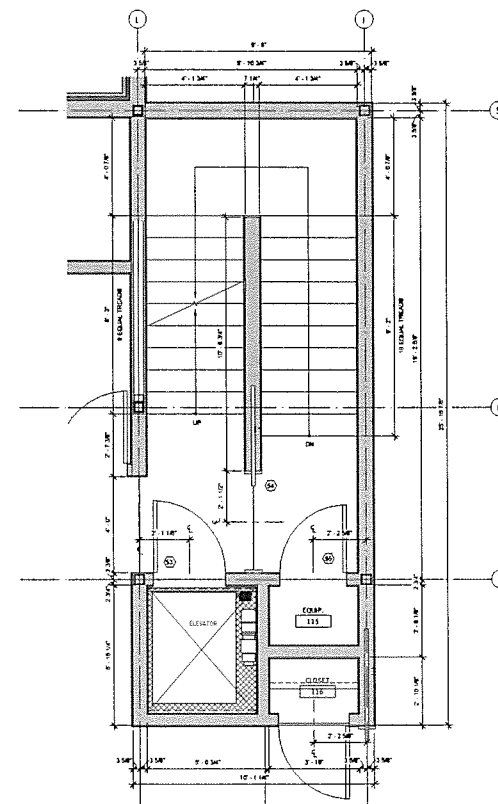
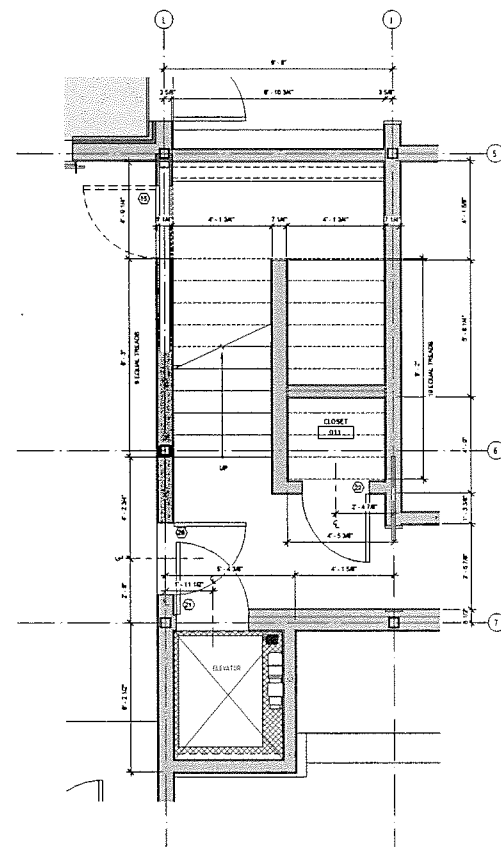
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Table 1 Demographic characteristics of study population

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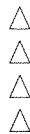
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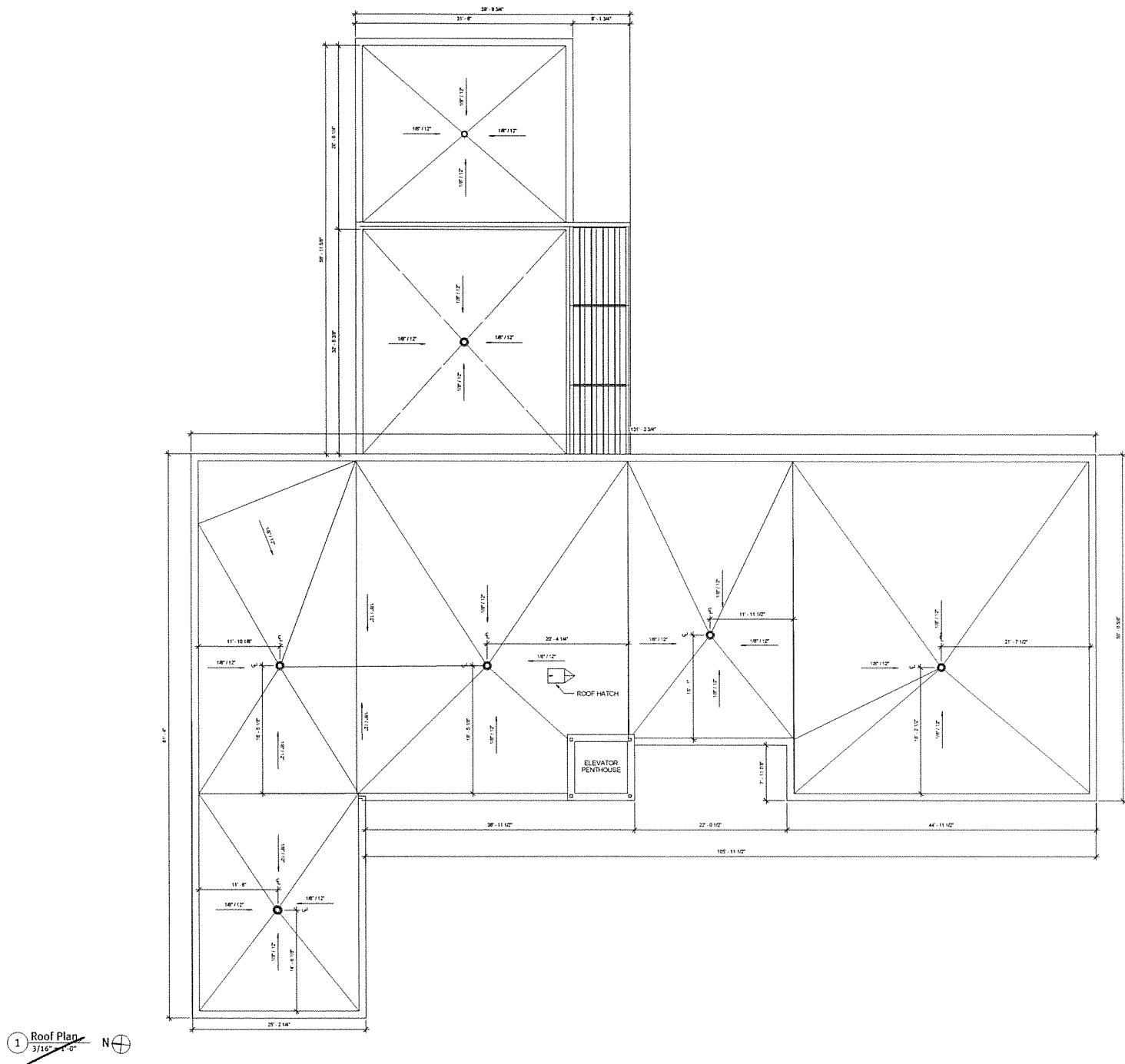
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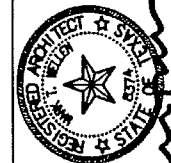
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$$1/16'' = 1'$$

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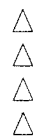
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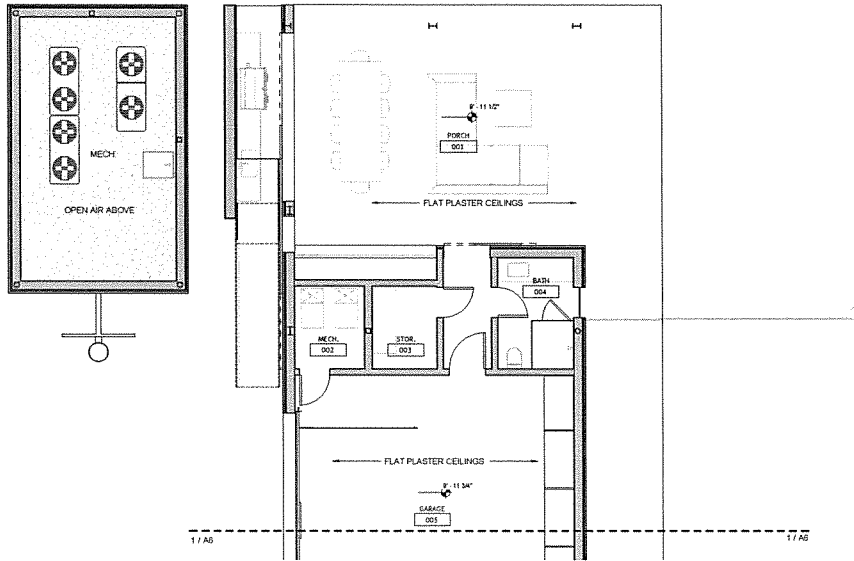
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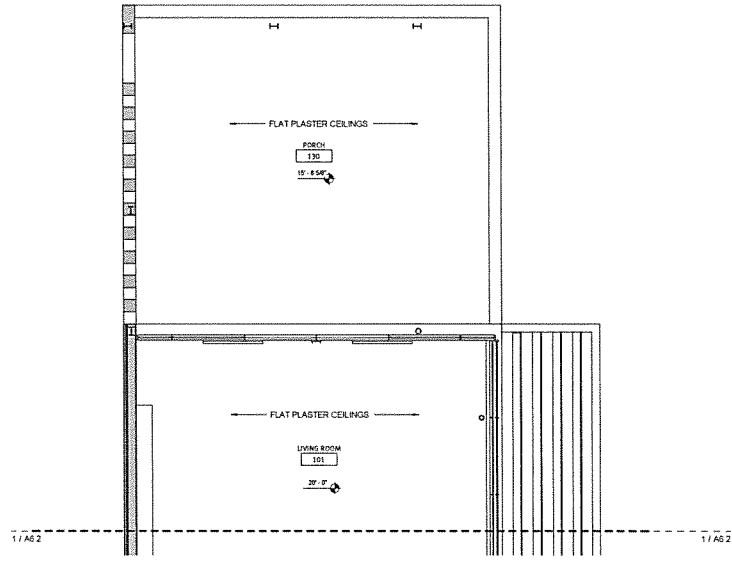
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2 BASEMENT - GARAGE & PORCH - RCP
1/4" = 1'-0"

1/16" = 1'

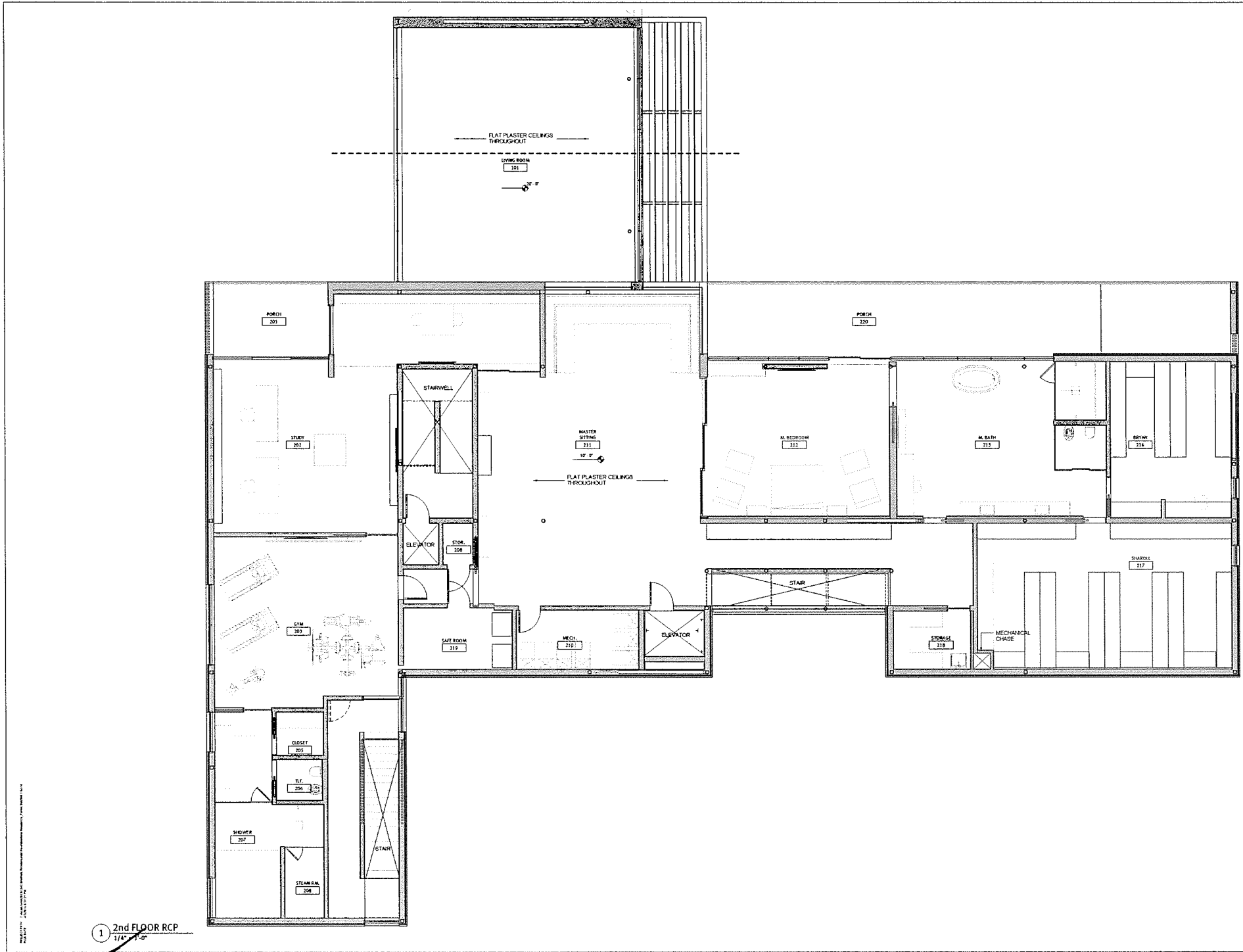


1 1st FLOOR - LIVING ROOM & PORCH - RCP
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1/16" = 1'

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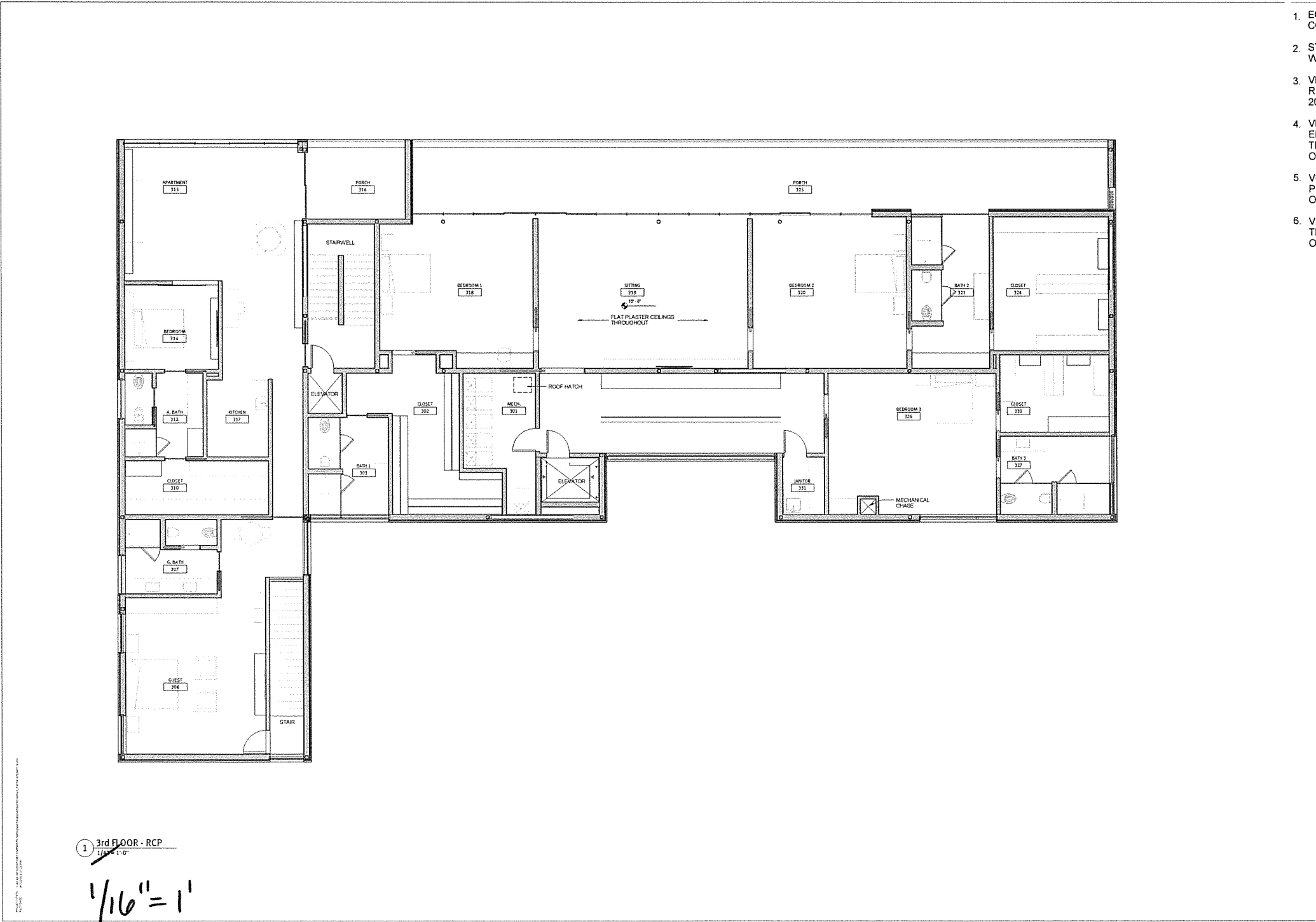
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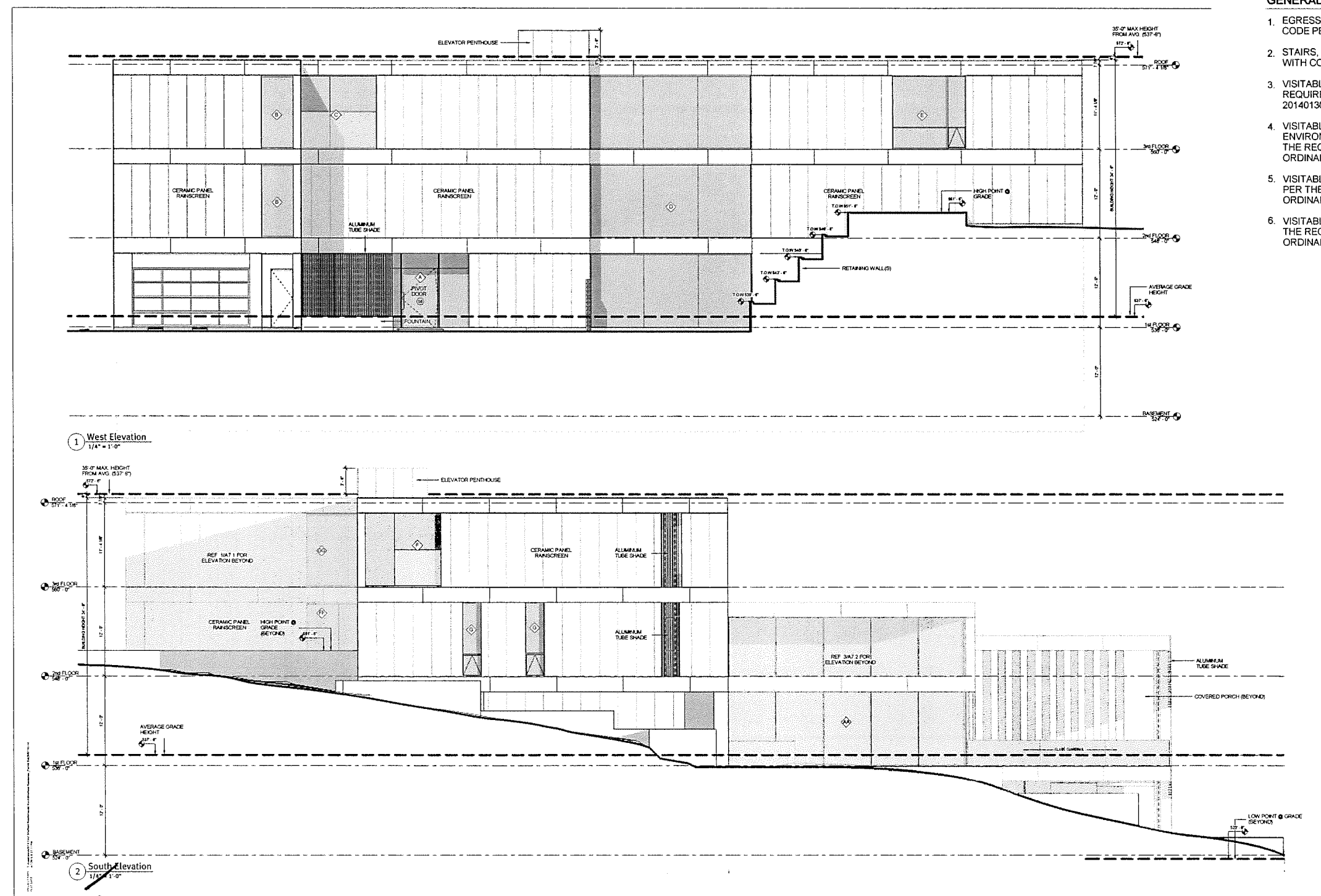
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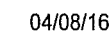
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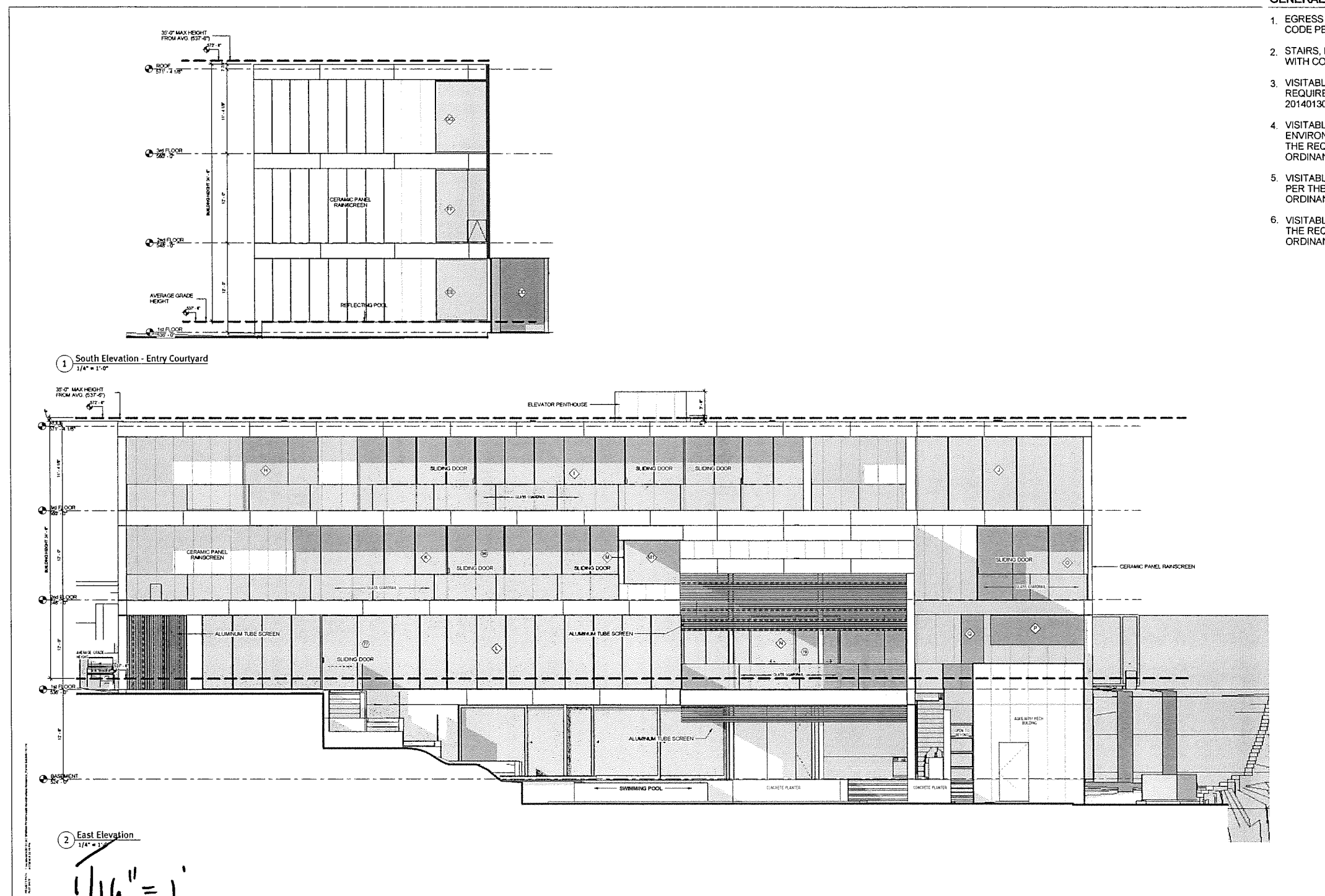
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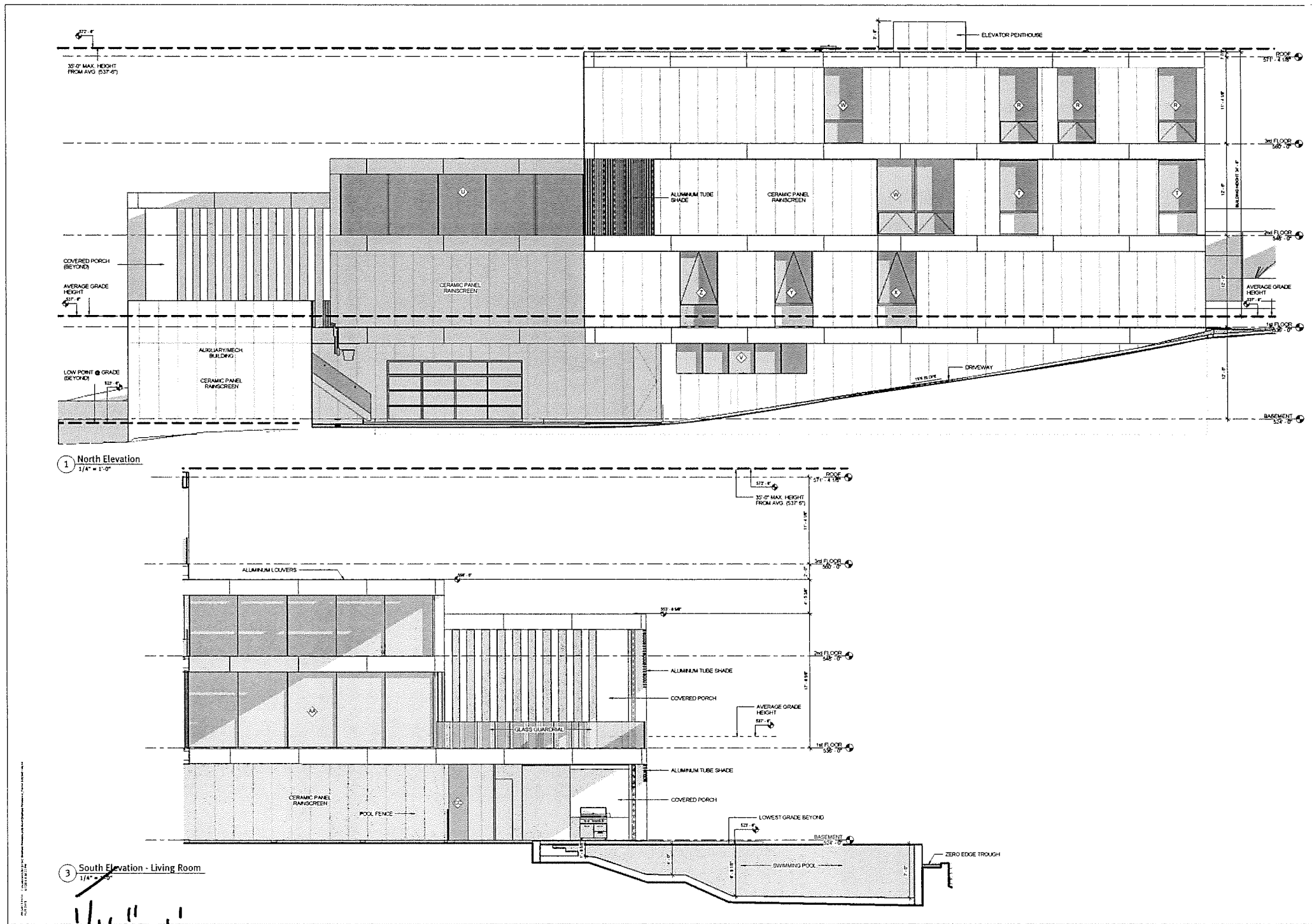
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Door Schedule									
Door Number	Door Size		Rough Width	Manufacturer	Frame Type	Details			Comments
	Width	Height				Head	Jamb	Sl	
1	4'-4 1/4"	10'-0"		Rimadesso - Velaria					
2	4'-0"	8'-0"		TBD					
3	3'-10"	10'-0"		TBD					
4	2'-6"	10'-0"		Lualaba - Rasomun555					
5	2'-6"	10'-0"		Lualaba - Rasomun555					
6	2'-8"	7'-11 1/2"		SHOWER					GLASS
7	3'-0"	10'-0"		Lualaba - Rasomun555					
8	2'-6"	10'-0"		Lualaba - Rasomun555					
9	3'-0"	10'-0"		Lualaba - Rasomun555					
10	2'-8"	10'-0"		Lualaba - Rasomun555					
11	2'-6"	10'-0"		Lualaba - Rasomun555					
12	3'-0"	10'-0"		Lualaba - Rasomun555					
13	3'-0"	10'-0"		TBD					
14	3'-0"	10'-0"		Lualaba - Rasomun555					
15	3'-0"	8'-0"		Lualaba - Rasomun555					
16	17'-10"	8'-0"	17'-10"	TBD					
17	2'-6"	10'-0"		Lualaba - Rasomun555					
18	2'-8"	10'-0"		Lualaba - Rasomun555					
19	2'-8"	10'-0"		Lualaba - Rasomun555					
20	3'-0"	10'-0"		Lualaba - Rasomun555					
21	3'-0"	8'-6"		Lualaba - Rasomun555					ELEVATOR
22	2'-10"	10'-0"		Lualaba - Rasomun555					
23	3'-4"	10'-0"		Lualaba - Rasomun555					
24	3'-0"	10'-0"		Lualaba - Rasomun555					
25	3'-0"	10'-0"		Lualaba - Rasomun555					
26	2'-8"	10'-0"		Lualaba - Rasomun555					
27	2'-8"	10'-0"		Lualaba - Rasomun555					
28	3'-0"	8'-6"		Lualaba - Rasomun555					ELEVATOR
29	36'-4 1/4"	10'-1 3/8"	36'-5 1/4"	Vitocsa					
30	26'-10"	10'-1 3/8"	26'-11"	Rimadesso - Velaria					
31	12'-9 3/8"	10'-0"		Lualaba - Rasomun555					
32	2'-10"	10'-0"		Lualaba - Rasomun555					
33	2'-10"	10'-0"		Lualaba - Rasomun555					
34	3'-0"	10'-0"		Lualaba - Rasomun555					
35	2'-8"	10'-0"		Lualaba - Rasomun555					
36	19'-10"	8'-0"	19'-10"	TBD					
37	3'-0"	7'-10 1/2"		TBD					
38	2'-8"	7'-0"		Lualaba - Rasomun555					
39	3'-0"	10'-0"		Lualaba - Rasomun555					
40	3'-0"	10'-0"		Lualaba - Rasomun555					
41	3'-0"	10'-0"		Lualaba - Rasomun555					
42	2'-8"	10'-0"		Lualaba - Rasomun555					
43	3'-0"	10'-0"		Lualaba - Rasomun555					
44	3'-8"	10'-0"		Lualaba - Rasomun555					
45	2'-8"	10'-0"		Lualaba - Rasomun555					
46	4'-6"	10'-0"		Lualaba - Rasomun555					
47	3'-0"	10'-0"		Lualaba - Rasomun555					
48	3'-8"	10'-0"		Lualaba - Rasomun555					
49	2'-6"	10'-0"		Lualaba - Rasomun555					
50	2'-8"	10'-0"		Lualaba - Rasomun555					
51	2'-8"	7'-11 1/2"		SHOWER					GLASS
52	3'-0"	10'-0"		Lualaba - Rasomun555					
53	3'-0"	8'-6"		Lualaba - Rasomun555					ELEVATOR
54	4'-0"	10'-0"		Lualaba - Rasomun555					
55	2'-10"	10'-0"		Lualaba - Rasomun555					
56	3'-0"	10'-0"		Lualaba - Rasomun555					
57	4'-6"	10'-0"		Lualaba - Rasomun555					
58	4'-11 1/4"	7'-11 5/8"		Vitocsa					
59	2'-8"	8'-6"	9'-5"	Lualaba - Rasomun555					
60	3'-0"	10'-0"		Lualaba - Rasomun555					
61	3'-0"	8'-6"		Lualaba - Rasomun555					
62	4'-0"	10'-0"		Lualaba - Rasomun555					
63	4'-0"	9'-6"		Lualaba - Rasomun555					
64	2'-6"	9'-6"		TBD					
65	2'-6"	7'-0"		TBD					
66	2'-6"	9'-6"		Lualaba - Rasomun555					
67	2'-6"	9'-6"		Lualaba - Rasomun555					
68	4'-0"	9'-6"		Lualaba - Rasomun555					
69	2'-6"	9'-6"		Lualaba - Rasomun555					
70	4'-3"	9'-6"		Lualaba - Rasomun555					
71	2'-8"	10'-0"		Lualaba - Rasomun555					
72	2'-8"	10'-0"		Lualaba - Rasomun555					
73	2'-11 3/8"	10'-0"		Vitocsa					
74	3'-0"	10'-0"		Lualaba - Rasomun555					
75	2'-6"	10'-0"	8'-1"	Lualaba - Rasomun555					
76	8'-7 3/4"	9'-10 3/8"		Rimadesso - Velaria					
77	8'-6 1/2"	10'-0"		Vitocsa					
78	7'-10"	10'-0"		Lualaba - Rasomun555					
79	12'-0 1/4"	10'-0"		Vitocsa					
80	2'-4"	7'-11 1/2"		SHOWER					GLASS
81	2'-8"	7'-11 1/2"		SHOWER					GLASS
82	2'-8"	10'-0"		Lualaba - Rasomun555					
83	2'-8"	10'-0"		Lualaba - Rasomun555					
84	3'-6"	10'-0"	7'-1"	Lualaba - Rasomun555					
85	3'-0"	10'-0"		Lualaba - Rasomun555					
86	4'-0"	10'-0"	8'-1"	Lualaba - Rasomun555					
87	8'-7 3/4"	9'-11 1/4"		Vitocsa					
88	3'-0"	10'-0"		Lualaba - Rasomun555					
89	2'-10"	10'-0"		Lualaba - Rasomun555					
90	4'-6"	10'-0"		Lualaba - Rasomun555					
91	2'-10"	10'-0"		Lualaba - Rasomun555					
92	2'-10"	10'-0"		Lualaba - Rasomun555					
93	3'-0"	10'-0"		Lualaba - Rasomun555					
94	12'-5"	10'-1 3/8"	12'-10"	Rimadesso - Velaria					
95	7'-6 1/2"	10'-0"		Vitocsa					
96	9'-0"	7'-10 1/2"		Lualaba - Rasomun555					
97	2'-8"	10'-0"		Lualaba - Rasomun555					
98	2'-8"	10'-0"		Lualaba - Rasomun555					
99	2'-8"	7'-11 1/2"		SHOWER					GLASS
100	2'-8"	10'-0"		Lualaba - Rasomun555					
101	2'-8"	10'-0"		Lualaba - Rasomun555					
102	2'-10"	10'-0"	8'-9"	Lualaba - Rasomun555					
103	8'-0 3/4"	10'-0"		Vitocsa					
104	3'-0"	10'-0"		Lualaba - Rasomun555					
105	2'-6"	10'-0"	8'-1"	Lualaba - Rasomun555					
106	2'-6"	10'-0"		Lualaba - Rasomun555					
107	2'-6"	7'-11 1/2"		SHOWER					GLASS
108	4'-0"	10'-0"	8'-1"	Lualaba - Rasomun555					
109	2'-8"	10'-0"	9'-5"	Lualaba - Rasomun555					
110	2'-8"	7'-11 1/2"		SHOWER					GLASS
111	2'-6"	10'-0"	8'-1"	Lualaba - Rasomun555					
112	2'-8"	10'-0"	9'-5"	Lualaba - Rasomun555					
113	8'-0"	8'-10"		Rimadesso - Velaria					
114	8'-7 1/2"	10'-0"		Vitocsa					
115	3'-0"	10'-0"	8'-1"	Lualaba - Rasomun555					
116	3'-0"	10'-0"		Lualaba - Rasomun555					
117	2'-8"	10'-0"	9'-5"	Lualaba - Rasomun555					
118	2'-6"	7'-11 1/2"		SHOWER					GLASS
119	2'-6"	7'-11 1/2"		SHOWER					GLASS
120	2'-6"	7'-11 1/2"		SHOWER					
121	3'-0"	10'-0"		Lualaba - Rasomun555					
122	7'-11 5/8"	10'-0"		Vitocsa					
123	3'-0"	10'-0"		Lualaba - Rasomun555					
124	5'-8"	7'-10 1/2"		Lualaba - Rasomun555					
125	3'-0"	10'-0"		Lualaba - Rasomun555					
126	3'-0"	10'-0"		Lualaba - Rasomun555					
127	3'-10 7/8"	10'-0"		Vitocsa					
128	3'-0"	10'-0"		Lualaba - Rasomun555					
129	3'-6"	10'-0"		Lualaba - Rasomun555					
130	2'-8"	10'-0"		Lualaba - Rasomun555					
131	2'-6"	7'-11 1/2"		SHOWER					GLASS
132	2'-6"	7'-11 1/2"		SHOWER					GLASS
133	2'-8"	10'-0"		Lualaba - Rasomun555					
134	2'-8"	10'-0"		Lualaba - Rasomun555					
135	2'-8"	10'-0"		Lualaba - Rasomun555					
136	2'-8"	7'-11 1/2"		SHOWER					GLASS
137	2'-6"	7'-11 1/2"		SHOWER					GLASS
138	3'-0"	10'-0"		Lualaba - Rasomun555					
139	3'-0"	10'-0"		Lualaba - Rasomun555					
140	17'-10"	8'-0"	17'-10"	TBD					
141	2'-6"	10'-0"		Lualaba - Rasomun555					
142	2'-10"	10'-0"		Lualaba - Rasomun555					
143	3'-0"	8'-0"		TBD					
144	2'-8"	10'-0"		Lualaba - Rasomun555					
145	2'-8"	10'-0"		Lualaba - Rasomun555					FUTURE DOOR

GENERAL NOTES

- EGRESS WINDOWS AND DOORS TO COMPLY WITH CODE PER 2012 IRC-R130.
- STAIRS, HANDRAILS, AND GUARDRAILS TO COMPLY WITH CODE PER 2012 IRC-R311 & R312.
- VISITABLE BATHROOM COMPLIES WITH THE REQUIREMENTS OF R320.3 OF THE COA ORDINANCE 20140130-021.
- VISITABLE LIGHT SWITCHES, RECEPTACLES, AND ENVIRONMENTS CONTROLS SHALL COMPLY WITH THE REQUIREMENTS OF R320.4 OF THE COA ORDINANCE 20140130-021.
- VISITABLE BATHROOM ROUTE WILL BE PROVIDED PER THE REQUIREMENTS OF R320.5 OF THE COA ORDINANCE 20140130-021.
- VISITABLE DWELLING ENTRANCE IS PROVIDED PER THE REQUIREMENTS OF R320.6 OF THE COA ORDINANCE 20140130-021.

NOTE: Sheet is formatted to 30"x48". Scales are 25% of noted when printed on 11x17 paper.

Westlake Residence
3701 Westlake Drive
Austin, TX

JOB NO. 1342

DRAWING TITLE

Basement

REVISIONS



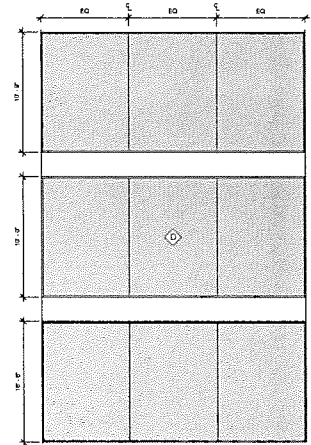
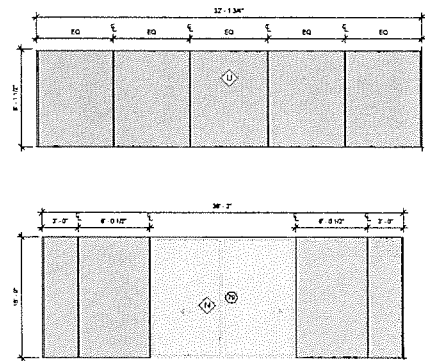
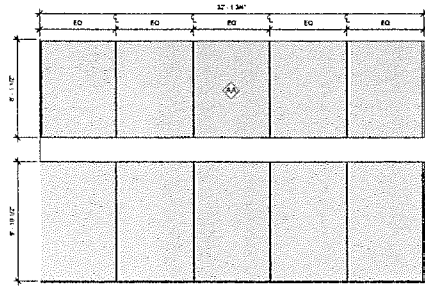
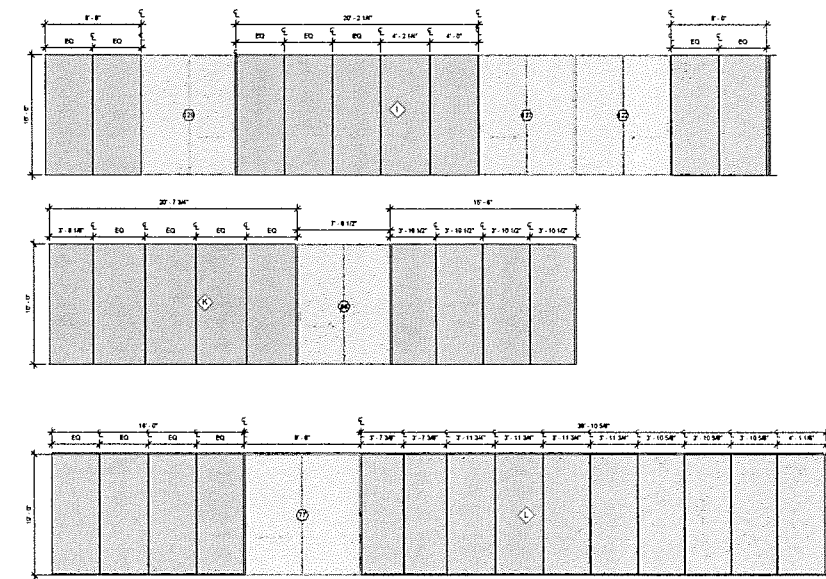
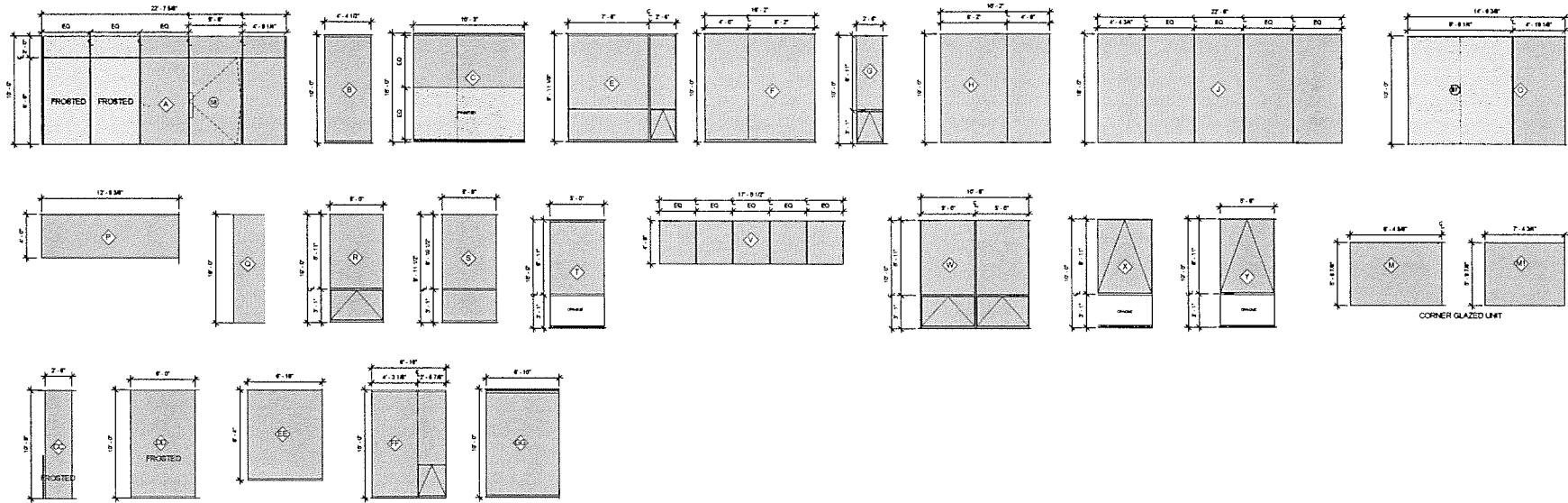
DATE 04/07/16

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GENERAL NOTES

1. EGRESS WINDOWS AND DOORS TO COMPLY WITH CODE PER 2012 IRC-R130.
2. DIMENSIONS SHOWN ARE TO FINISH FACE. ROUGH OPENING DIMENSION WILL BE DETERMINED UPON WINDOW MANUFACTURER SPECS.
3. GLAZING TYPE TO BE UNIFORM THROUGHOUT HOUSE. VERIFY GLAZING MANUFACTURER WITH ARCHITECT.
4. OPAQUE OR FROSTED GLAZING TO BE VERIFIED WITH ARCHITECT.

NOTE: Sheet is formatted to 30"x48". Scales are 25% of noted when printed on 11x17 paper.

Westlake Residence
3701 Westlake Drive
Austin, TX

JOB NO. 1342

DRAWING TITLE

Basement

REVISIONS

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SHEET NO.

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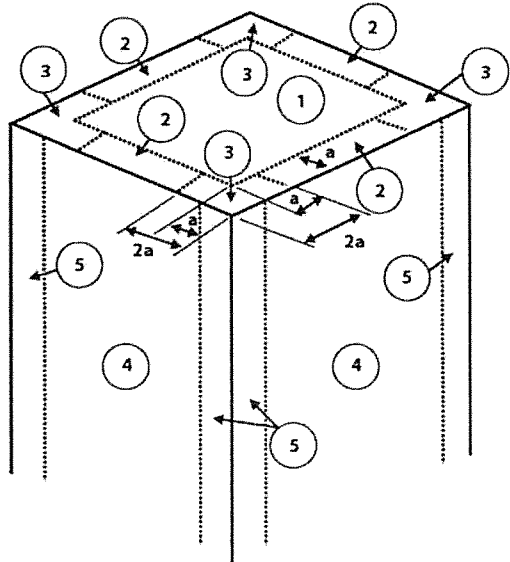
1102 West Texas Avenue
Midland, Texas 79701
432.682.1252
432.682.1257 fax

COORDINATION

- Only large openings in structural framing members are shown on the structural drawings. However, all sleeves, embeds, inserts, openings and frames that are necessary for the work shall be provided. The Contractor shall coordinate with all trades sizes, locations and placement. All openings and embedded items which have an effect on the structure shall be submitted to the Engineer for review.
- Refer to Architectural, Mechanical, Electrical and Plumbing drawings for floor elevations, location of depressed or elevated floor areas, slopes and drains.
- Contractor shall coordinate the requirements for building equipment supported on or from the structure. Submittals identify all equipment including size, dimensions, clearances, accessibility, weights and reactions. Any deviations from specified equipment shall be noted on the submittals.
- Shop drawings shall be prepared for all structural items and submitted for review by the Engineer. Contract Drawings shall not be reproduced and used as shop drawings. All items deviating from the Contract Drawings or from previously submitted shop drawings shall be noted.
- The details designated as "Typical Details" apply generally to the Drawings in all areas where conditions are similar to those described in the details.
- The design and provision of all temporary supports required for the execution of the contract such as guys, braces, shores, reshores, falsework, supports and anchors are not included in these drawings and shall be the responsibility of the Contractor. Temporary supports shall not result in the overstress or damage to the structure.

CODES

- IBC 2012 International Building Code with City of Austin Ordinance No. 20130606-089 Amendments.
- Structural Concrete: Building Code Requirements for Structural Concrete, American Concrete Institute, ACI 318-11.
- Structural Steel: Steel Construction Manual, American Institute of Steel Construction, Thirteenth Edition. Specification for Structural Steel Buildings, AISI 360-10.



DESIGN LOADS

- Live Loads
 - Mechanical Rooms 125 psf
 - Storage (minimum) 125 psf
 - Roof 20 psf
 - Garages (passenger vehicles only) 40 psf
 - Passenger Vehicle load 3000 lbs
 - Single Family Residential: 40 psf
 - Balconies 40 psf
 - Stairs 40 psf
 - Partition at areas with 80 psf live load or less 15 psf
 - Awnings or canopies 20 psf

- Dead Loads include the self weight of the structural elements and the following superimposed loads:
 - Ceiling and Mechanical at roof 10 psf
 - Ceiling and Mechanical at floor 5 psf
 - Roofing and rigid insulation 15 psf

- Wind Loads
 - Wind Lateral Load on Structural Frame is based on the following:
 - Ultimate Design 3-sec. gust Wind Speed, V_{ult} 115 mph
 - Wind Importance Factor, I 1.00
 - Wind Exposure B
 - Internal Pressure Coefficient, GC_{pi} ± 0.18
 - Component & Cladding Design Pressures:

Effective Area:	$\leq 10ft^2$	(Overhangs)
Zone 1	+14.9 psf; -23.6 psf	+10.9 psf; -19.7 psf
Zone 2	+14.9 psf; -41.1 psf	+10.9 psf; -48.1 psf
Zone 3	+14.9 psf; -60.8 psf	+10.9 psf; -80.9 psf
Zone 4	+23.6 psf; -25.6 psf	
Zone 5	+23.6 psf; -31.5 psf	

Effective Area:	$50ft^2$	(Overhangs)
Zone 1	+11.8 psf; -22.1 psf	+10.0 psf; -18.2 psf
Zone 2	+11.8 psf; -33.5 psf	+10.0 psf; -48.1 psf
Zone 3	+11.8 psf; -51.6 psf	+10.0 psf; -62.6 psf
Zone 4	+21.2 psf; -23.2 psf	
Zone 5	+21.2 psf; -26.6 psf	

Effective Area:	$> 100ft^2$	(Overhangs)
Zone 1	+10.5 psf; -21.4 psf	+10.0 psf; -17.5 psf
Zone 2	+10.5 psf; -30.2 psf	+10.0 psf; -48.1 psf
Zone 3	+10.5 psf; -47.7 psf	+10.0 psf; -54.7 psf
Zone 4	+20.1 psf; -22.1 psf	
Zone 5	+20.1 psf; -24.5 psf	

- NOTE: Zone 1 = Interior roof area not in Zones 2 or 3.
Zone 2 = Perimeter roof area not in Zone 3 (8'-0" strips).
Zone 3 = Corner roof areas where Zone 2 strips intersect.
Zone 4 = Wall areas not in Zone 5.
Zone 5 = Wall corner areas (8'-0" strips).

- Calculate the effective area for each component & cladding element, as defined by ASCE 7, depending on length and location. Effective area shall be the maximum of the following:

- Effective Area = Length x Tributary Width (OR) Length x (Length/3)
- Interpolation of uplift pressures is allowed between effective areas.

- Roof Snow Loads
 - Ground Snow Load 5 psf

5. Earthquake Loads

- Seismic Lateral Load on Structural Frame is based on the following:
 - Seismic Importance Factor, I 1.00
 - Occupancy Category II
 - Mapped Spectral Response Accelerations
 - S_s 0.063g
 - S_1 0.033g
 - Site Class B
 - Spectral Response Coefficients
 - S_{DS} 0.042g
 - S_{D1} 0.022g
 - Seismic Design Category A
 - Basic Seismic-Force-Resisting System: Steel Ordinary Concentrically Braced Frame
 - Design Base Shear 133 kips
 - Seismic Response Coefficient, C_s 0.013
 - Response Modification Factor, R 3.25
 - Analysis Procedure Equivalent Lateral Force

- Structural elements supporting elevators are designed based on published data for the following elevator types:
 - Eclipse Home Elevator - Type # - [#]
- Loading for mechanical rooms and kitchens are based on the weights of equipment and concrete pads as indicated on the contract documents. Any revisions in equipment type, size, or quantity shall be reported to the Architect immediately for verification of the structural design.
- Floor live loads noted above have been reduced in accordance with the building code at the rate of .15 percent per square foot in excess of 150 square feet. Roof live load has not been reduced.

BUILDING MOVEMENTS

- The building movements specified herein are anticipated to occur and shall be taken into account by the Contractor in the design, detailing, and installation of the building elements.
- Spandrel beam deflections: Provisions shall be made in the building cladding for relative floor to floor vertical deflections of smaller of span/600 or $\frac{1}{8}$ ".
- Interior floor/roof deflections: Provisions shall be made in interior partitions and other elements supported by or attached to the floors or roofs for relative floor to floor vertical deflections of span/240.
- Lateral building drift: Provisions shall be made in building cladding and other architectural finishes for relative floor to floor lateral deflections of story height/400.

TESTING LABORATORY SERVICES

- Work specified herein shall be performed by a qualified independent Testing Laboratory, selected and paid by the Owner.
- Pier drilling operation: Make continuous inspections to determine that the proper bearing stratum is obtained and utilized for bearing and that the shafts are clean and dry before pouring concrete.
- Filling and Backfilling operation:
 - Analyze backfill samples delivered by the contractor to determine compliance with gradation and quality requirements of the geotechnical report.
 - Make in place compaction tests for moisture content, moisture density relationship, and density of materials in place. Perform one test for each 5000 square feet of area per lift.
- Footing excavation: Inspect the excavations to determine that the proper bearing stratum is obtained and utilized for bearing and that excavations are properly clean and dry before concrete is placed.

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nava
ARCHITECTS

aecc
ARCHITECTS



Sen-Gin Liu
04.08.2016

Sheffield Residence
3509 Westlake Drive
Austin, TX

JOB NO. 00000
DRAWING TITLE
GENERAL NOTES
REVISIONS
DATE 11/12/15
DRAWN BY NK, JC
CHECKED BY RL
SHEET NO.

S001

5. Concrete inspection and testing:

- a. Secure composite samples of concrete at the jobsite in accordance with ASTM C172.
b. Mold and cure three specimens from each sample in accordance with ASTM C31. Test specimens in accordance with ASTM C39. Two specimens shall be tested at 28 days for acceptance and one shall be tested at seven days for information.
c. Perform one strength test (four cylinders) for each pour.
d. Make one slump test for each set of cylinders following the procedural requirements of ASTM C143 and C172.
e. Determine total air content of air entrained concrete in accordance with ASTM C231. Perform one test for each strength test.

6. Concrete Reinforcement: Inspect all concrete reinforcing steel and embedded metal assemblies prior to placement of concrete for compliance with Contract Documents and shop drawings. All instances of non-compliance shall be immediately brought to the attention of the contractor for correction, and if uncorrected, reported to the engineer.

7. Expansion Anchors: Provide continuous inspection of expansion bolt installation to ensure that holes are of the specified size, and that bolts are properly installed including application of minimum installation torques.

8. Structural steel, steel joists, and joist girders: Field inspection of proper erection of all members, visual examination of all field welding, visual inspection of all bolts, inspection of all shop fabricated members upon arrival at the jobsite for conformance with accepted fabrication and erection drawings, verification of welder's certificates.

EXCAVATION PROTECTION

1. The sides of all excavations greater than 5'-0" in depth shall be laid back to a slope of 1 horizontal to 1 vertical, unless the following applies:
- a. A steeper slope is allowed by the geotechnical engineer for the particular location and site conditions in question.
b. A retention system is indicated on the Contract Drawings.
c. An alternative protective system is submitted by the Contractor and allowed by the Owner.
2. Contractor shall submit Drawings and calculations sealed by a Registered Engineer licensed in the State of Texas for the design of any alternative protective systems. Alternative protective systems shall be designed to resist the soil pressures stipulated in the project geotechnical report prepared by Holt Engineering, Inc., dated July 20, 2015. In addition, the design shall consider surcharges created by construction equipment, excavation spoil, and other surface encumbrances.
3. Contractor shall comply with all Occupational Safety and Health Administration standards and all other regulatory agency standards regarding excavation safety.

BUILDING PAD PREPARATION

1. Structural fill material shall have a plasticity index between 3 and 18. Gradation of material shall be as follows:
- | | |
|-------------------------------|-----------|
| Retained on 2 1/2" screen | 0% |
| Retained on 1 1/2" screen | 0% - 25% |
| Retained on 3/4" screen | 15% - 55% |
| Retained on 1/2" screen | 45% - 75% |
| Retained on No. 40 mesh sieve | 60% - 90% |
2. Prior to placing fill material, remove all organic and other deleterious material from the existing subgrade for a distance of 3'-0" beyond building line. All exposed surfaces shall then be scarified to a depth of _____, watered as required and recompacted to a minimum of 95 percent of the maximum dry density as defined by ASTM D 698 (Standard Proctor Test) at a moisture content within ±3 percent of the optimum moisture content.
3. Structural fill shall be placed in 8 inch loose lifts, watered as required and compacted to a minimum of 95 percent of the maximum dry density as defined in ASTM D 698 at a moisture content within ±3 percent of the optimum moisture content.
4. Compaction and moisture content of subgrade and each lift of structural fill shall be inspected and approved by a qualified engineering technician, supervised by a Geotechnical Engineer.

5. Provide a 1 foot thick clay cap at perimeter of building to protect pad from moisture intrusion. Cap can be formed of on-site clays, placed in 6" lifts, and compacted to 90 percent of the maximum dry density as defined by ASTM D 698 (Standard Proctor Test). Cap shall slope away from building.
6. Slab on grade shall be placed over a minimum of 1 ft. structural fill, except where slab areas are excavated down to limestone rock. In these areas, the slab is soil supported on a gravel capillary barrier consisting of a minimum of 6 inches of 3/8" diameter (pea size) uniformly graded washed gravel.
7. Provide a 10 mil polyolefin vapor barrier. Place vapor barrier in accordance with manufacturer's recommendation on top of structural fill.
8. Building pad preparation information is based on a geotechnical report provided by Holt Engineering, Inc dated July 20, 2015.

CONTROLLED BACKFILL

1. Backfill material shall have a plasticity index between 3 and 18.
2. Fill shall be placed in lifts not to exceed 6 inches.
3. Fill shall be compacted at the optimum moisture content (±3%) to between 90 and 95 percent of the maximum dry density per ASTM D698.
4. Compaction and moisture content of controlled backfill shall be verified by an independent testing laboratory.
5. The top 1 ft of material below the ground surface shall consist of relatively impervious material, with a liquid limit between 40 and 50 percent and a plasticity index between 20 and 30. This material shall be placed in 6" lifts and compacted at optimum moisture content, to 95 percent of the maximum density per ASTM D698.
6. Backfill material shall not be placed against foundation walls until all supporting slabs, beams, struts, etc., have attained their 28 day design strength unless proper bracing is installed.
7. Where backfill is required on both sides of a structure or building element, backfill shall be placed simultaneously along both sides so that the backfill height on one side does not exceed the height on the opposite side by more than 4'-0".
8. Design of basement and retaining walls is based on equivalent hydrostatic pressure of 45 pcf, assuming free draining backfill and use of perforated drain pipe in accordance with the geotechnical report prepared by Holt Engineering, Inc. dated July 20, 2015.

DRILLED PIERS

1. Pier design is based on an allowable loading of 15,000 psf in end bearing and 1,250 psf in side friction in accordance with the geotechnical report dated July 20, 2015 by Holt Engineering, Inc.
2. Bearing stratum shown on the pier detail 1/S302, is tan limestone.
3. Piers not specifically located on the plan shall be located on centerline of column above. Where no column occurs, locate on centerline of wall or beam.
4. Provide dowels from piers into concrete above using same bar size and number as shown for pilaster above. Where no pilaster occurs, use dowels of same size and number as pier reinforcing steel. Extend dowels 30 bar diameters into pier and beam, wall, pilaster or column u.n.o.
5. Elevation of top of piers, unless noted otherwise on the drawings is at the bottom of the deepest intersecting beam or wall supported by the pier.
6. Reinforcing cage shall be held securely away from earth at sides and bottom by sets of 3 spacers at a maximum spacing of 8-ft. along the length of the cage and 1'-0" from the bottom.
7. Pier reinforcing and concrete shall be placed immediately after drilling operations are complete; in no case shall a pier be drilled that cannot be poured by the end of the workday.

8. See plans for pier sizes, reinforcing, and depth.
9. The contractor shall verify depths of piers before pier steel is cut. Pier steel may be delivered to the jobsite in standard lengths and cut as required. Provide 64 bar diameter laps in all vertical pier reinforcing.
10. Reinforcing steel shop drawings shall include placing drawings for templates to set dowels in piers.
11. Top of pier shall be of the specified diameter. Form top of pier if required to maintain the specified diameter. Any concrete extending beyond the specified diameter shall be removed.
12. Temporary steel casing may be required during pier drilling operations. Prior to the placement of concrete, any seepage water shall be removed from the pier holes. Special construction procedures in accordance with ACI 336.1-98 and ACI 336.3R-98 and specifications shall be followed during extraction of the casing and during concrete placement.
13. Contractor shall include in bid documents, unit-costs for casing if required and unit-cost for greater and lesser depth of drilling for each pier size. Base bids shall be for cased piers.
14. All piers shall be inspected by a representative of Holt Engineering, Inc. in order to ensure that the proposed bearing material has been reached in accordance with the recommendations given in the geotechnical report.
15. The contractor shall make and maintain accurate records of the drilled pier depths, bearing stratum, depth of penetration into bearing stratum, diameter and location (including off center eccentricities), and shall submit this information to the Engineer.

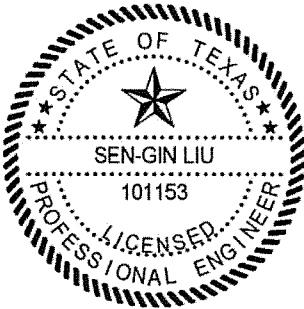
CONCRETE FOOTINGS

1. Concrete footing design is based on an allowable net bearing capacity of 8,000 psf in accordance with the geotechnical report dated July 20, 2015 by Holt Engineering, Inc.
2. Bearing stratum shown on the footing details is tan limestone.
3. Footings not specifically located on the plan shall be located on centerline of pilaster or column above. Where no pilaster or column occurs, locate on centerline of wall or beam.
4. Provide dowels from footings into concrete above using same bar size and number as shown for pilaster or column above. Where no pilaster or column occurs, use 4-#7 dowels. Extend dowels 30 bar diameters into pier and wall, beam, pilaster or column u.n.o.
5. Elevation of top of plinths/footings, unless noted otherwise on drawings, is at the bottom of the deepest intersecting beam or wall supported by the footing.
6. Footing excavations shall be to neat lines and shall be free of loose or wet materials.
7. Footing reinforcing and concrete shall be placed immediately after excavations are complete; in no case shall a footing be excavated that cannot be placed by the end of the workday.
8. See plans and schedules for footing sizes, reinforcing and depths.
9. Reinforcing steel shop drawings shall include placing drawings for templates to set dowels in footings.
10. All footings shall be inspected by a representative of Holt Engineering, Inc. in order to ensure that the proposed bearing material has been reached in accordance with the recommendations given in the geotechnical report and that the footing has been constructed to specified size, with detailed reinforcing, and to specified tolerances.

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CAST IN PLACE CONCRETE

1. Cast in place concrete shall meet the following requirements:

Class	28 Day Strength	Aggregate Type	Size	Slump	Use
A	4000 psi	NWC33	1"	5"-7"	Piers
B	4000 psi	LWC330	¾"	3"-5"	Conc. on metal deck
C	4000 psi	NWC33	1"	3"-5"	Slabs on grade, grade beams and footings

2. Fly ash shall not be used in architecturally exposed concrete.
3. Provide 5 percent plus or minus 1½ percent of entrained air in concrete permanently exposed to the weather and elsewhere at the contractors option. Do not use entrained air in drilled piers.
4. Horizontal construction joints in concrete pours shall be permitted only where indicated on the drawings. All vertical construction joints shall be made in the center of spans in accordance with the typical details. Contractor shall submit proposed locations for construction joints not shown on drawings for review by the Architect and Structural Engineer. Additional construction joints may require additional reinforcing as specified by the Engineer which shall be provided by the contractor at no additional cost to the owner.
5. Embedded conduits, pipes, and sleeves shall meet the requirements of ACI 318-11, Section 6.3, including the following:
- a. Conduits and pipes embedded within a slab, wall, or beam (other than those passing through) shall not be larger in outside dimension than ½ the overall thickness of the slab, wall or beam in which they are embedded.
- b. Conduits, pipes and sleeves shall not be spaced closer than three diameters or widths on center.
6. Concrete pours shall not exceed 5000 square feet or 100 linear feet on each side without prior approval by the Architect for each pour.

SLAB ON GRADE

1. See plan and detail 14/S3.01 for slab on grade pour strips.
2. Provide control joints or construction joints at the centerlines of all columns and at 15 feet on center maximum in both directions. Provide additional joints such that the resulting aspect ratio does not exceed 1:1.5
3. Tooled, sawcut, or preformed joints shall be ¼ the depth of the slab. Sawcut joints must be made within 12 hours after the slab has been placed.
4. Metal keyway forms or bulkheads shall be removed prior to placement of adjacent pours.
5. Refer to "Building Pad Preparation" section for fill requirements.
6. Erection equipment that imposes any concentrated load in excess of 2,000 lbs acting over a 2'-6"x2'-6" area may not be used on the slab-on-grade. Equipment used that will exceed this loading shall be staged away from the building slab and means for doing so shall be included in base bids.
7. Notes 1-3 Do not apply to the 10" structural slabs on grade, refer to drawings for location.

CONCRETE REINFORCING

1. Reinforcing steel shall be deformed new billet steel bars in accordance with ASTM A615 Grade 60.
2. Detailing of reinforcing steel shall conform to the American Concrete Institute Detailing Manual.

3. All hooks and bends in reinforcing bars shall conform to ACI detailing standards unless shown otherwise.
4. Provide reinforcing bars in accordance with the bar bending diagram if bar types are specified. In unscheduled beams, slabs, columns and walls detail reinforcing as follows:
- a. Lap top reinforcing bars at mid span.
- b. Lap bottom reinforcing bars at the supports.
- c. Lap vertical bars in columns and walls only at floor lines, unless noted otherwise.
- d. Refer to lap splice schedule for splice length requirement.
- e. Reinforcement labeled as continuous shall be lap spliced 38 bar diameters as a minimum, unless otherwise noted.
- f. Provide standard hooks in top bars at cantilever and discontinuous ends of beams, walls and slabs.
- g. Provide corner bars for all horizontal bars at the inside and outside faces of intersecting beams or walls. Corner bars are not required if top, bottom, or horizontal bars are hooked.
5. Welding of reinforcing steel will not be permitted.
6. Heat shall not be used in the fabrication or installation of reinforcement.
7. Reinforcing steel clear cover shall be as follows:

- a. Grade beams - 1½" top, 3" bottom, 2" side (formed), 3" side (placed against earth)
- b. Drilled piers - 3" bottom, 3" sides
- c. Walls - Refer to Details

CONCRETE JOINT SEALANT

1. Concrete joint sealant includes routing, sawcutting, surface preparation and application of waterproof concrete joint sealant. Sealant used where exposed to pedestrian or vehicle traffic shall be suited for traffic. Repair deteriorated concrete adjacent to crack or joint as required.
2. Joint sealing shall be performed by workers qualified to perform the work. As a minimum, the foreman shall have not less than two years experience with structural concrete repairs.
3. Joints shall be sealed with a waterproof concrete joint sealant product from one of the following manufactureres (or an equivalent product submitted to the engineer for approval):
- a. Sto
- b. Euclid
- c. Master Builders
- d. Thoro
- e. Dayton Superior
4. Existing concrete shall be prepared as recommended by the manufacturer including but not limited to the following:
- a. Remove any existing joint sealant from crack or joint
- b. Saw cut or route if necessary to clean joint
- c. Repair damaged concrete as required
5. Apply joint sealant in accordance with the manufacturer's directions.
6. Apply sealant within working time limits and temperatures identified by the manufacturer.

EPOXY MORTAR

1. Work related to epoxy mortar shall be performed in accordance with ACI 503.4.

2. Epoxy mortar shall comply with ASTM C881, Type III. If the proposed mortar is not a prepackaged product, submit the proposed mortar mix to the testing agency for review and approval. Epoxy mortars or mortar resins offered by the following manufacturers are acceptable:
- a. Sika Corporation
- b. Euclid Chemical Company
- c. Master Builders
3. All surfaces to receive mortar shall be free of all loose and unsound material, oil, grease, wax, or other bond inhibiting agents. Use sandblast or waterblast to clean surface. Acid etching shall not be used.
4. Mix, place and compact mortar in accordance with manufacturer's recommendations.
5. Finish mortar to match texture and color of surrounding concrete. Provide test samples as required until adequate match can be demonstrated.
6. All surfaces shall be inspected by the testing agency prior to mortar application to ensure that surfaces have been properly prepared.

EXPANSION ANCHORS

1. Expansion anchors shall only be used where specified on the drawings. The contractor shall obtain approval from the engineer of record prior to using the anchors for missing or misplaced cast-in-place anchors.
2. Unless otherwise noted, size and depth of the expansion anchors specified in the drawings are based on the Hilti Fastening System products - Hilti Kwik Bolt 3 for general applications, and Kwik Bolt TZ for overhead applications.
3. Substitution of expansion anchor products with similar capacities shall be submitted to the engineer of record for approval.
4. Expansion anchors of the size and embedment shown on the Drawings shall be installed in accordance with the Contract Documents, the manufacturer's recommendations, and the manufacturer's current ICBO report for the anchor. If conflicts exist between these referenced documents, the most stringent requirements shall govern.
5. The Contractor shall locate all existing reinforcing steel and other embedded items contained in the concrete using non-destructive methods and shall position anchor locations to avoid conflicts with existing embedded items. Anchor locations can be adjusted by a maximum of 1½" from detailed locations to avoid conflicts, unless noted otherwise.
6. Based on field verified locations of reinforcing steel and embedded items, the Contractor shall create templates for each anchor group. Submit template dimensions for review prior to fabrication of connection plates.
7. Holes for anchors shall be drilled in a continuous operation using the bit type and size recommended by the anchor manufacturer. Holes shall be drilled perpendicular to the concrete surface and shall not be enlarged or redirected at any point along its length. All debris shall be blown out of the holes with compressed air after drilling.
8. All abandoned holes shall be filled with non-shrink grout.
9. Holes in connection plates shall be no more than ⅛" larger than the anchor diameter. If larger holes are required for erection purposes, Contractor shall provide ¼" x 3" x 3" plate washers sufficiently welded to the connection plate to transfer the specified load.
10. Installation of expansion anchors shall be continuously inspected by the testing agency to ensure that holes are of specified size, and that bolts are properly installed including application of minimum installation torques.

ADHESIVE ANCHORS

1. Adhesive anchors shall only be used where specified on the drawings. The contractor shall obtain approval from the engineer of record prior to using the anchors for missing or misplaced cast-in-place anchors.
2. Unless otherwise noted, size and depth of the adhesive anchors specified in the drawings are based on HAS rods epoxy doweled with HY 200, Hilti Fastening Systems.

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DATE 04/08/16
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SHEET NO. S003

3. Substitution of expansion anchor products with similar capacities shall be submitted to the engineer of record for approval.
4. Adhesive anchors of the size and embedment shown on the Drawings shall be installed in accordance with the Contract Documents, the manufacturer's recommendations, and the manufacturer's current ICBO report for the anchor. If conflicts exist between these referenced documents, the most stringent requirements shall govern.
5. The Contractor shall locate all existing reinforcing steel and other embedded items contained in the concrete using non-destructive methods and shall position anchor locations to avoid conflicts with existing embedded items. Anchor locations can be adjusted by a maximum of 1 inch from detailed locations to avoid conflicts, unless noted otherwise.
6. Based on field verified locations of reinforcing steel and embedded items, the Contractor shall create templates for each anchor group. Submit template dimensions for review prior to fabrication of connection plates.
7. Holes for anchors shall be drilled in a continuous operation using the bit type and size recommended by the anchor manufacturer. Holes shall be drilled perpendicular to the concrete surface and shall not be enlarged or redirected at any point along its length. All debris shall be blown out of the holes with compressed air after drilling.
8. All abandoned holes shall be filled with non-shrink grout.
9. Holes in connection plates shall be no more than $\frac{1}{16}$ " larger than the anchor diameter. If larger holes are required for erection purposes, Contractor shall provide $\frac{1}{4}$ " x 3" x 3" plate washers sufficiently welded to the connection plate to transfer the specified load.
10. Installation of adhesive anchors shall be continuously inspected by the testing agency to ensure that holes are of specified size, and that bolts are properly installed.

ADHESIVE DOWELS

1. Adhesive dowelling system shall be one of the following products: Hilti "HY 200", Hilti "RE 500-SD" epoxy system. Install dowels in accordance with the manufacturer's instructions.
2. Clean out holes with compressed air after drilling holes.
3. Prior to drilling holes for dowels, locate existing reinforcing steel with a Pachometer (R-Meter) or by drilling $\frac{1}{8}$ " diameter pilot holes. Relocate bolt holes as required to avoid existing reinforcement.
4. Abandoned holes shall be completely filled with adhesive dowelling compound.

STRUCTURAL STEEL

1. Structural Steel shall conform to ASTM A992 or A572, grade 50 except where A36 is noted on plan, except that miscellaneous plates, angles, and channels may be A572, grade 50 or A36. Steel pipe shall conform to ASTM Specification A 501 or ASTM A 53, Type E or S, Grade B. Steel tube shall conform to ASTM Specification A 500, Grade B, Fy 46 ksi.
2. Anchor rods shall conform to ASTM F1554 grade 36 ksi.
3. Column base plates shall be grouted with a non-shrink, high strength nonmetallic grout conforming to ASTM C827, and shall have a compressive strenght at 28 days of 5000 psi. Pre-grouting of base plates will not be permitted.
4. Studs shall be Nelson studs type S3L (Fu=65 ksi) or acceptable equal. Studs shall be made from cold drawn steel conforming to ASTM A108.
5. Deformed bar anchors shall be Nelson D2L or KSM deformed bar anchors (or acceptable equal) and shall be made from cold drawn wire per STM A490 conforming to ASTM A108 with minimum yield strength of 70 ksi. Anchors shall be automatically and welded with suitable welding equipment in the shop or in the field. Welding shall be in accordance with the recommendations of Nelson Stud Company or KSM Welding Company.
6. Structural steel detailing, fabrication, and erection shall conform to the AISC "Specification for Steel Buildings" and the AISC "Code of Standard Practice for Steel Buildings and Bridges". Typical connection details are indicated in the drawings. The fabricator shall prepare drawings based on these details. If alternate connection designs are used, the fabricator shall have a registered professional engineer prepare the

connection designs. Such connection shall bear the engineer's seal and shall be submitted with shop drawings.

7. Splicing of structural steel members is prohibited without prior approval of the Engineer as to location and type of splice to be made. Any member having splice not shown and detailed on shop drawings will be rejected.
8. All welds denoted as moment connection or full penetration weld shall be ultrasonically or x-ray certified by an independent testing agency.
9. Contractor shall coordinate structural steel fireproofing requirements. All interior structural steel, including steel joists, scheduled or indicated to receive spray applied fireproofing shall be delivered to the project site unprimed. Steel exposed to corrosive conditions after installation shall be primed with a protective coating which does not diminish the bond between the spray applied fireproofing, and the steel substrate. Any primer, and/or coating applied to structural steel shall be approved for use in the applicable U.L. Fire Resistance Assembly used on the project. Contractor shall protect any unprimed structural steel from detrimental effects of corrosion, as required, until the steel is enclosed and protected by the new construction.
10. Shop painting: Paint structural steel with one coat of manufacturer's standard red oxide primer applied at a rate to provide a uniform dry film thickness of 2.5 mils.
11. Contractor must fabricate and erect steel in accordance with OSHA Safety requirements, 29 CF part 1926 Safety for Steel Erection, Final Rule.

STRUCTURAL STEEL CONNECTIONS

1. Welding shall conform to ANSI/AWS D1.1, latest edition.
2. Bolts conform to ASTM A325. Bolts shall be designed using values for bearing type bolts with thread allowed in the shear plane.
3. Structural steel connections not specifically detailed on the Drawings shall be designed and detailed by the Contractor under the direct supervision of a registered engineer licensed in the State of Texas. Sealed calculations for all connections designed by the Contractor shall be submitted for the Architect's files. Connections that meet the requirements and assumptions presented in our schematic connection details and table can be used at the discretion of the Contractor. The Contractor shall take full responsibility in confirming that the connection tables are used within their limitations and assumptions outlined in the details and notes.
4. Beam connections shall be designed and detailed as follows, unless noted otherwise on the Drawings:
- a. Connections shall be AISC type 2 simple framing connections.
- b. In general, shop connections shall be bolted or welded and field connections shall be bolted.
- c. Where indicated, connections shall be designed for the scheduled shear force, the shear force indicated on the Drawings as "V=", and the horizontal force indicated as "H=".
- d. If not indicated on the Drawings, connections shall be designed for 65 percent of the total load capacity for the beam span shown in the beam tables in Part 3 of the AISC Manual, fourteenth edition.
- e. The minimum number of rows of bolts shall be $\frac{1}{6}$ of the beam depth with any fraction be rounded to the next higher number.
- f. Bolts shall be "snug tight", U.N.O.
- g. Short slotted holes shall be permitted provided washers are installed in accordance with AISC requirements. Washers shall be hardened where A325 bolts are utilized. Long slotted holes are not permitted unless the connection is designed as slip-critical or as specified otherwise.
5. Wind brace and truss connections shall be designed and detailed as follows, unless noted otherwise on the Drawings:
- a. Connections shall be welded.
- b. Connections shall be designed and detailed for the forces shown on the Drawings.
- c. If forces are not indicated on the Drawings, connections shall be designed to develop the full tensile capacity of the members.
6. For connections not specifically addressed by these notes or the Drawings, provide fillet welds at all contact surfaces sufficient to develop the tensile strength of the smaller member at the joint.

7. Moment connections indicated on Drawings as $\blacktriangleleft\blacktriangleright$ shall be welded to develop the full capacity of the member on both sides of supporting member.
8. Roof edges angles shall be continuous and shall be spliced only at supports. Splices shall be butt-welded to develop full capacity of the member.
9. Fillet welds with no size specified shall be $\frac{3}{16}$ ", or minimum size required by AISC, whichever is larger.

COMPOSITE STEEL BEAM NOTES

1. Composite steel beams do not require shoring to support the weight of the wet concrete.
2. Headed studs shall be $\frac{1}{2}$ " diameter x 3 $\frac{1}{2}$ " long.
3. Place the number of studs shown on plan as follows:
- a. Uniform distribution: for beams where only a total number of studs is specified place studs at a uniform spacing between the ends of the beam.
- b. Segmented distribution: for beams supporting other beams, with studs noted between beam reactions and/or the ends of the beam, place studs at a uniform spacing between beam reactions and/or the beam ends.
4. Headed studs shall be placed in a single row where possible. Where additional studs are required beyond those that can be placed in a single row, place studs in pairs starting towards the nearest end of the beam. Refer to stud placement diagram in typical details. The contractor shall be responsible for determining the proper headed stud layout for each beam prior to installation of the studs in the field.
5. Minimum number of studs are called out on plan. Additional studs may be needed to meet the maximum stud spacing requirements shown on the shear stud placement diagram.

COMPOSITE METAL DECK AND CONCRETE SLAB

1. Composite metal deck shall be 2" deep, 20 gauge deck, Type VLI with an 12" rib pattern. Minimum positive section modulus shall be 0.341 in³ per linear foot of deck. Minimum negative section modulus shall be 0.346 in³ per linear foot of deck. Min. Yield strength shall be 50 ksi.
2. Composite metal deck shall be galvanized per ASTM A525, class G60 coating.
3. Total slab thickness consisting of composite metal deck and concrete fill shall be 4 $\frac{1}{2}$ ".
4. Deck shall be continuous over 4 or more supports so as not to require any intermediate shoring to support construction loads and wet concrete, unless noted otherwise. Two span deck shall not be used unless approved by EOR. Contractor may provide properly designed heavier gauge deck installed in single span lengths if desired to eliminate shoring requirements.
5. Reinforce the slab over the composite deck with W2.1xW2.1 WWM chaired to be located 1" below the top of the slab. Provide a 6'-0" wide extra layer of mesh over all interior beams and girders spanning parallel to the deck span.
6. See detail 4/S504 for metal edge forms.
7. Slabs over composite deck shall be placed and finished to provide a floor within specified tolerances. Slab thicknesses may vary away from the columns due to deflections of the beams and deck.

METAL ROOF DECK

1. Steel Roof Deck shall be 1 1/2" deep, type B, 20 gauge deck with 36" coverage. Minimum positive section modulus shall be 0.234 in³ per linear foot of deck. Minimum negative section modulus shall be 0.247 in³ per linear foot of deck.
2. Roof deck shall be galvanized with a class G60 coating.
3. Roof deck shall be continuous over four or more supports.

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4. Roof deck connections shall be as follows:

- a. Install powder-actuated and screw fasteners according to the manufacturer's recommendations.
- b. Powder-actuated fasteners shall be manufactured from AISI modified steel, austempered to a Rockwell C Hardness of 52-58. Screw frame fasteners shall be manufactured from Grade 1010 to 1022 to 10B08 to 10B22 carbon steel per STM A510.
- c. Powder-actuated fasteners shall have full-tip knurled shanks and minimum 12-mm diameter steel washers. Screw frame fasteners shall have wave form cutting edge self-drilling tips and Hex Washer Heads.
- d. Powder-actuated and screw frame fasteners shall be zinc plated to a thickness of 5mm in accordance with ASTM B633, Sc. 1 Type III.
- e. Powder-actuated and screw frame fasteners shall be SDI listed for diaphragm design and uplift, UL and FM listed for fire resistance and wind uplift. Sidelap connectors shall be FM listed for wind uplift.
- f. 36 inch wide deck sheets shall be attached to all deck supports using 4 approved Hilti frame fasteners (a fastener at each sidelap and at every other flute). Fasteners shall be Hilti X-HSN 214 or X-ENP-19 powder-actuated fasteners or Hilti S-MD 12-24x1-5/8 HWH5 screw fasteners. At wind braces, sheets shall be fastened to deck supports using 7 fasteners per sheet (a fastener at each sidelap and at every flute).
- g. Approved sidelap connectors shall be Hilti S-SLC 01 M HWH or S-SLC 02 M HWH sidelap.
- h. The installer that will be using the tools to attach the powder-actuated the powder-actuated frame fasteners shall be trained and certified by fastener manufacturer's representative on the general use of powder-actuated technology and fastening guidelines for the attachment of steel deck. The installer that will be using the tools to attach the screw fasteners shall be trained by fastener manufacturer's representative on the proper tools and fastening guidelines for the attachment of steel deck.
- i. Within 15 ft. from the building perimeter, the deck sheets shall be fasteners to all supports at each flute.
- j. At side laps: 2 sidelap connectors equally spaced at 3 equal spaces between supports
5. Power driven fasteners shall be selected by the contractor for the combinations of deck gauge and deck support member thickness. Submit proposed fasteners with complete manufacturer's information, including diaphragm shear values for Engineer's review.
6. Ceilings, mechanical, electrical, & plumbing systems shall not be supported by the metal roof deck.
7. Submittal: Submit deck layout plans and details indicating deck type, fastening methods and layout, support locations, projections, openings and reinforcement, and any other pertinent details and accessories.

STRUCTURAL ABBREVIATIONS

ADDITIONAL ADD'L
ADJACENT ADJ.
AGGREGATE AGGR.
ALTERNATE ALT.
ANCHOR ROD A.R.
ARCHITECT(URAL) ARCH('L)
AIR CONDITIONER A/C
AIR HANDLING UNIT AHU
APPROXIMATE(LY) APPORX.
AXIAL LOAD P

BACK FACE B.F.
BEAM BM.
BEARING BRG.
BETWEEN BTWN.
BLOCKING BLKG.
BLOCK-OUT B.O.
BOTTOM BOT.
BOTTOM OF STEEL B.O.S.
BRICK LEDGE BR.L.
BRIDGING BRDG.
BUILDING BLDG.
BUILDING LINE B.L.
CAST-IN-PLACE C.I.P.
CENTER LINE C.L. OR C.
CENTER LINE OF STEEL C.L.S.
CENTER OF GRAVITY C.G.
CLEAR(ANCE) CLR.
COLUMN COL.
COMPRESSION C OR COM
CONCRETE CONC.
CONCRETE MASONRY UNIT C.M.U.
CONNECTIONS CONX(S)
CONTINUOUS CONT.
CONTRACTOR CONTR.
CONTROL JOINT CT.J.
CONSTRUCTION CONST.
CONSTRUCTION JOINT C.J.
COVER PLATE COV. PL.

DEFORMED BAR ANCHOR(S) DBA('S)
DETAIL DET.
DEAD LOAD D.L.
DIAGONAL DIAG.
DIAMETER DIA.
DIMENSION(S) DIM(S)
DRAWING(S) DWG(S)
DOUBLE DBL.
DOUBLE EXTRA STRONG XXS
DOWEL(S) DWL(S)

EACH EA.
EACH FACE E.F.
EACH WAY E.W.
ELECTRICAL ELEC.
ELEVATION EL.
ELEVATOR ELEV.
EMBEDMENT EMBED.
ENGINEER ENGR.
EQUAL EQ.
EQUIPMENT EQUIP.
EXPANSION EXP.
EXPANSION JOINT E.J.
EXISTING EXIST.
EXTERIOR EXT.
EXTRA STRONG XS

FACE TO FACE F. TO F.
FABRICATE(ION)(OR) FAB.
FAR SIDE F.S.
FINISHED FIN('D)
FINISHED FLOOR F.F.
FIREPROOF F.P.
FLANGE FLG.

FACE TO FACE F. TO F.
FABRICATE(ION)(OR) FAB.
FAR SIDE F.S.
FINISHED FIN('D)
FINISHED FLOOR F.F.
FIREPROOF F.P.
FLANGE FLG.

FLOOR FL.
FLOOR DRAIN F.D.
FOOTING FTG.
FOUNDATION FDN.

GALVANIZED GALV.
GENERAL GEN.
GLUE LAMINATED TIMBER GLULAM
GRADE GR.
GRADE BEAM GR.BM.

HOT DIP(PED) H.D.
HEADED STUD(S) H.S.
HEADER HDR.
HEIGHT HT.
HORIZONTAL HORIZ.
HOOK HK.

INSIDE DIAMETER I.D.
INSIDE FACE I.F.
INTERIOR INT.
INTERMEDIATE INTERM.

JOINT JT.
JOIST(S) JST(S).

LAMINATED VENEER LUMBER LVL
LIGHTWEIGHT LWT.
LIVE LOAD L.L.
LONGITUDINAL LONG.
LONG LEG HORIZONTAL LLH.
LONG LEG VERTICAL LLV.
LONG SIDE HORIZONTAL LSH.
LONG SIDE VERTICAL LSV.

MANUFACTURE(R) MFR.
MASONRY MAS.
MATERIAL MAT'L
MECHANICAL MECH('L)
METAL MTL.
MEZZANINE MEZZ.
MIDDLE MID.
MISCELLANEOUS MISC.
MOMENT M.
MOMENT CONNECTION(S) M.C.

NEAR FACE N.F.
NOMINAL NOM.
NON-SHRINK N.S.
NORMAL WEIGHT N.W.
NOT IN CONTRACT N.I.C.
NOT TO SCALE N.T.S.

ON CENTER O.C.
OPENING(S) OPNG(S).
OPPOSITE OPP.
OPPOSITE HAND O.H.
ORIENTED STRAND BOARD OSB
OUTSIDE FACE O.F.
OUTSIDE DIAMETER O.D.

PARALLEL PAR.
PARALLEL STRAND LUMBER PSL
PENETRATION PEN.
PERPENDICULAR PERP.
PIECE PC.
PLATE PL. OR PL.
PLYWOOD PLYWD.

POINT PT.
POST-TENSION(ED) P.T.
POUND(S) X1000 KIP(S)
POUNDS PER LINEAR FOOT PLF
POUNDS PER SQUARE FOOT PSF
POUNDS PER CUBIC FOOT PCF
POUNDS PER CUBIC YARD PCY
PRECAST CONCRETE P/C
PREFABRICATED PREFAB.
PRELIMINARY PRELIM.
PRESSURE PRESS.

PROJECT(ION) PROJ.
RADIUS R.
REFER TO / REFERENCE REF.
REINFORCED CONCRETE PIPE RCP.
REINFORCE(ING)(ED)(MENT) REINF.
REMAINDER REM.
REQUIRE REQ.
REQUIRED REQ'D
RETURN RET.
ROOF DRAIN R.D.
ROUGH OPENING R.O.
ROUND RND.

SCHEDULE(D) SCHED.
SECTION SECT.
SHEAR FORCE V.
SHEET SHT.
SIMILAR SIM.
SPACE(S)(ING) SPA.
SPECIFICATION(S) SPEC(S).
SPECIFIED SPEC'D
SQUARE SQ.
STAINLESS STEEL S.S.
STANDARD STD.
STEEL STL.
STIFFENER STIFF.
STRAIGHT STR.
STIRRUPS STIR.
STRUCTURE OR STRUCTURAL STRUCT.
SUPPORT(S) SUPT(S)

TENSION T.
THICK(NESS) THK.
TONGUE AND GROOVE T&G
TOP AND BOTTOM T&B
TOP OF BEAM T.O.BM.
TOP OF FOOTING T.O.FTG.
TOP OF PIER T.O.PIER
TOP OF PIER CAP T.O.P.C.
TOP OF STEEL T.O.S.
TOP OF STRUCTURAL CONCRETE T.O.S.C.
TOP OF WALL T.O.W.
TREATED TRTD.
TYPICAL TYP.

UNLESS OTHERWISE NOTED U.O.N.
VERTICAL VERT.
VOLUME VOL.

WATER STOP W.S.
WELDED WIRE MESH W.W.M.
WIND BRACE WB
WIND LOAD W.L.
WITH W/
WITHOUT W/O
WATER PROOFING W.P.
WORK POINT W.P.
WOOD WD.

MATERIALS LEGEND

EXISTING CONSTRUCTION

CONCRETE

STEEL IN SECTION

PLYWOOD IN SECTION
CMU
BRICK OR STONE IN SECTION
GROUT/SAND
EARTH (UNDISTURBED)
EARTH/FILL (COMPACTED)
ROCK
MECH. UNIT OR ZONE

DRAFTING SYMBOLS

DIRECTION OF VIEW FOR SECTION CUT OR ELEVATION
SECTION NUMBER
SHEET NUMBER

SECTION MARK

DIRECTION OF VIEW

ELEVATION MARK

SPECIFIC LOCATION DESCRIBED BY DETAIL

DETAIL MARK

PLAN/DETAIL DESIGNATION

PLAN NAME/DETAIL TITLE

SCALE:

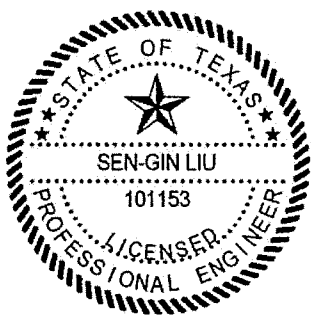
STRUCTURAL DRAWING TYPES

S0 ... GENERAL NOTES
S1 ... PLANS/FOUNDATION CONSTRUCTION
S3 ... CONCRETE CONSTRUCTION
S5 ... STEEL CONSTRUCTION

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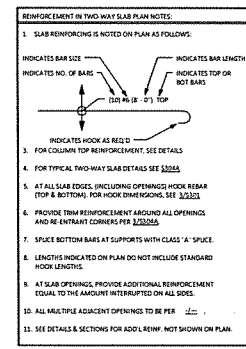
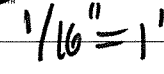
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09.08.2016

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GENERAL NOTES
REVISIONS

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SHEET NO.

S005

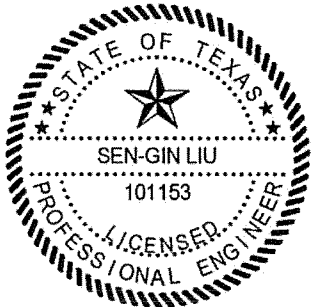
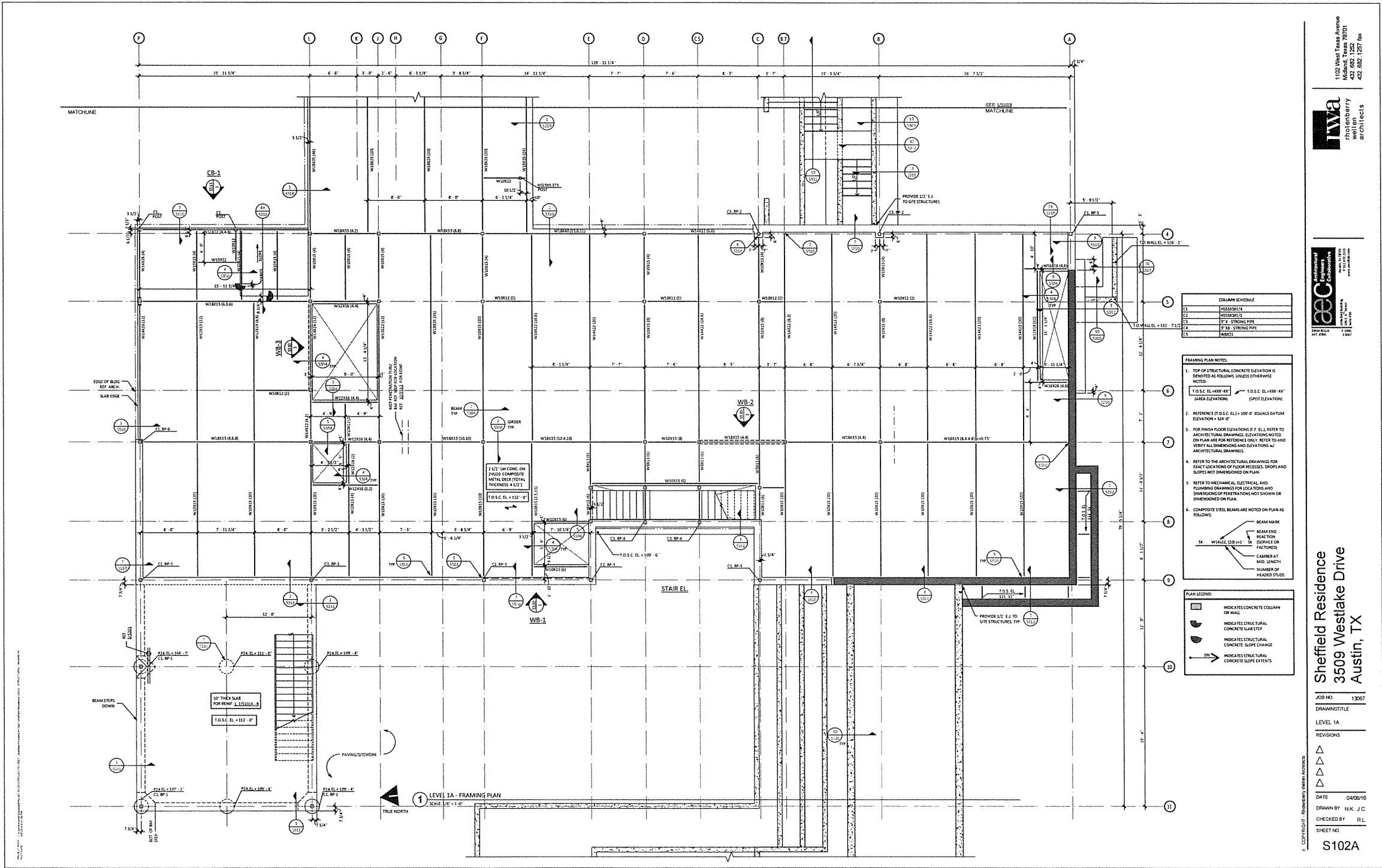

$$1/16'' = 1'$$


Aer. Jui Li
04.08.2016

△
△
△
△

DATE 04/08/10
DRAWN BY N.K., J.C.
CHECKED BY R.L.
SHEET NO.

S101B - F



Sen-Gin Liu
04.08.2016

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Professional Engineer
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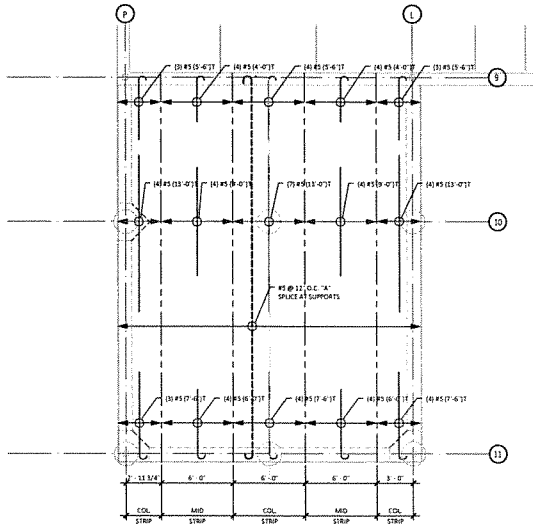
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DRAWING TITLE
LEVEL 1A
REVISIONS
DATE 04/08/16
DRAWN BY N.K. J.C.
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SHEET NO.
S102A

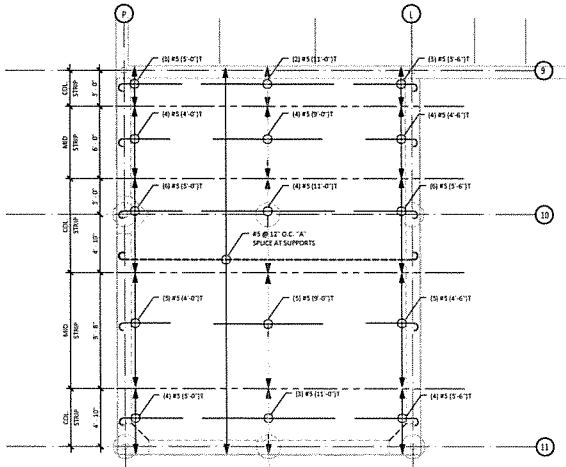
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1 LEVEL 1A - FRAMING PLAN E-W.
SCALE: 1/4" = 1'-0"
TRUE NORTH

1/16" = 1'



2 LEVEL 1A - FRAMING PLAN N-S.
SCALE: 1/4" = 1'-0"
TRUE NORTH



REINFORCEMENT IN TWO-WAY SLAB PLAN NOTES:	
1. SLAB REINFORCEMENT IS NOTED ON PLAN AS FOLLOWS:	
INDICATES BAR SIZE	INDICATES BAR LENGTH
INDICATES NO. OF BARS	INDICATES TOP OR BOT BARS
INDICATED HOOK AS RED D	
2. FOR COLUMN TOP REINFORCEMENT, SEE DETAILS	
3. FOR TYPICAL TWO-WAY SLAB DETAILS SEE 22036	
4. AT ALL SLAB EDGES, INCLUDING OPENINGS, HOOK REBAR (TOP & BOTTOM). FOR HOOK DIMENSIONS, SEE 22032	
5. PROVIDE REINFORCEMENT AROUND ALL OPENINGS AND RE-ENTRANT CORNERS PER 22036	
6. SPICE BOTTOM BARS AT SUPPORTS WITH CLASS "X" SPICE	
7. LENGTHS INDICATED ON PLAN DO NOT INCLUDE STANDARD HOOK LENGTHS	
8. AT SLAB OPENINGS, PROVIDE ADDITIONAL REINFORCEMENT EQUAL TO THE MAXIMUM INTERCEPTED ON ALL SIDES	
9. ALL MULTIPLE ADJACENT OPENINGS TO BE PER 22036	
10. SEE DETAILS & SECTIONS FOR ADDL REINF. NOT SHOWN ON PLAN	

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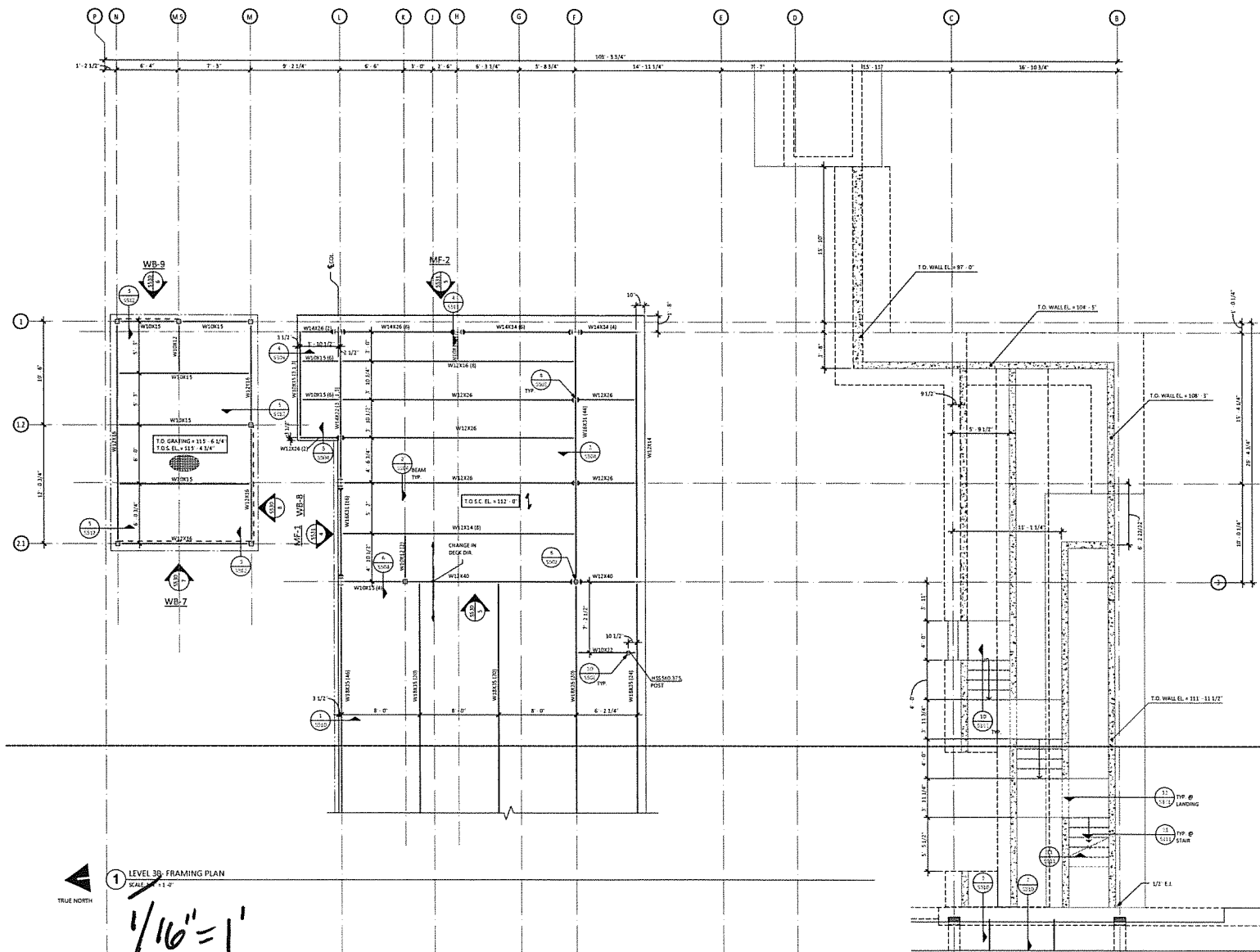
JOB NO. 13067
DRAWING TITLE
LEVEL 1A TWO-WAY
SLAB REINFORCEMENT
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DRAWN BY N.K. J.C.
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SHEET NO.

S102A - R

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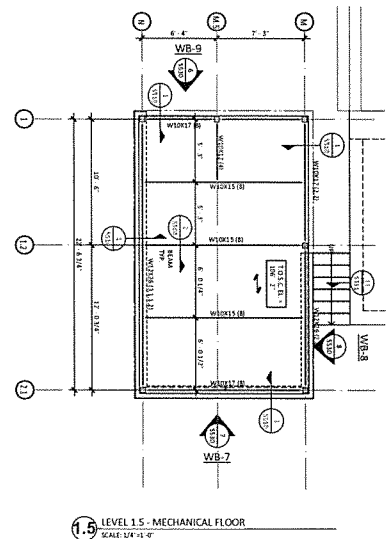


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- FRAMING PLAN NOTES:**
1. TOP OF STRUCTURAL CONCRETE ELEVATION IS IDENTIFIED AS FOLLOWS, UNLESS OTHERWISE NOTED:
T.O.S.C. EL. +XXX.XX' (DATA ELEVATION) **T.O.S.C. EL. +XXX.XX'** (SPOT ELEVATION)
 2. REFERENCE (T.O.S.C. EL.) + 100'-0" EQUALS DATUM ELEVATION + 204'-0".
 3. FOR FINISH FLOOR ELEVATIONS, IF F.L.L. REFER TO ARCHITECTURAL DRAWINGS. ELEVATIONS NOTED ON PLAN ARE FOR REFERENCE ONLY. REFER TO AND VERIFY ALL DIMENSIONS AND ELEVATIONS IN ARCHITECTURAL DRAWINGS.
 4. REFER TO THE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS OF FLOOR RECESSES, SLOOPS AND SLOPES NOT DIMENSIONED ON PLAN.
 5. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR LOCATIONS AND DIMENSIONS OF PENETRATIONS NOT SHOWN OR DIMENSIONED ON PLAN.
 6. COMPOSITE STEEL BEAMS ARE NOTED ON PLAN AS FOLLOWS:
SK BEAM MARK
W12X12 BEAM END REACTION (SERVICE OR FACTORED)
SK CAMBER AT MID-LENGTH
NUMBER OF HEADED STUDS

- PLAN LEGEND:**
- INDICATES CONCRETE COLUMN OR WALL
 - INDICATES STRUCTURAL CONCRETE SLAB STOP
 - INDICATES STRUCTURAL CONCRETE SLOPE CHANGE
 - INDICATES STRUCTURAL CONCRETE SLOPE EXTENT



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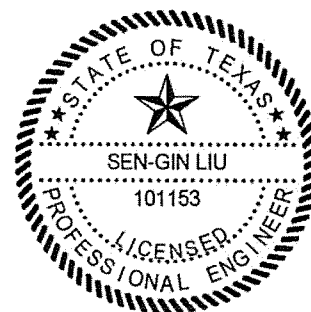
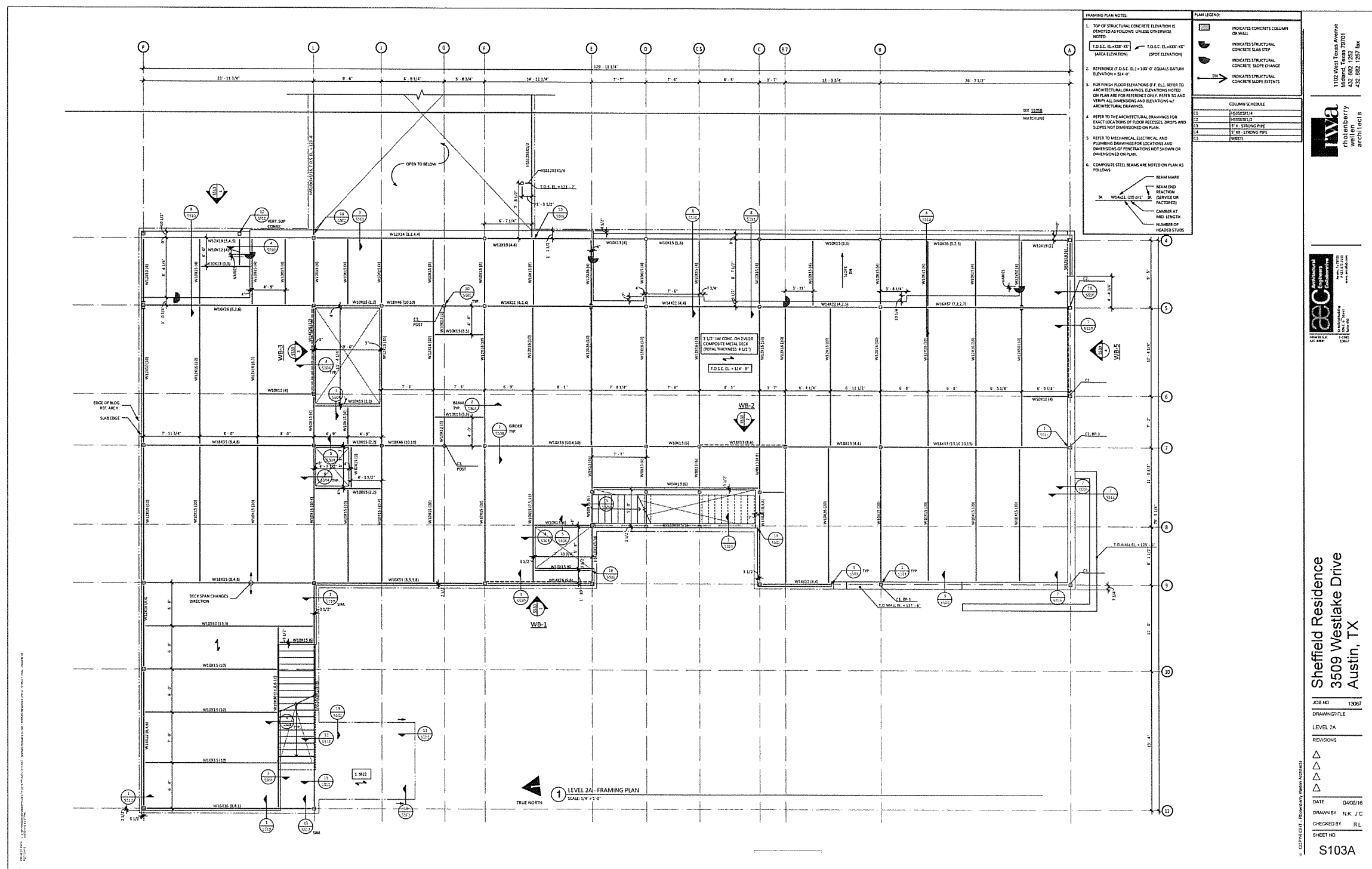
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Sen-Gin Liu
04.08.2016

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CHECKED BY R.L.
SHEET NO. S102B



Aer. Jim Li
04.08.2011


$$1/16'' = 1'$$

DATE 10/10/2018
BY 2103A

**Architectural
Engineers
Collaborative**

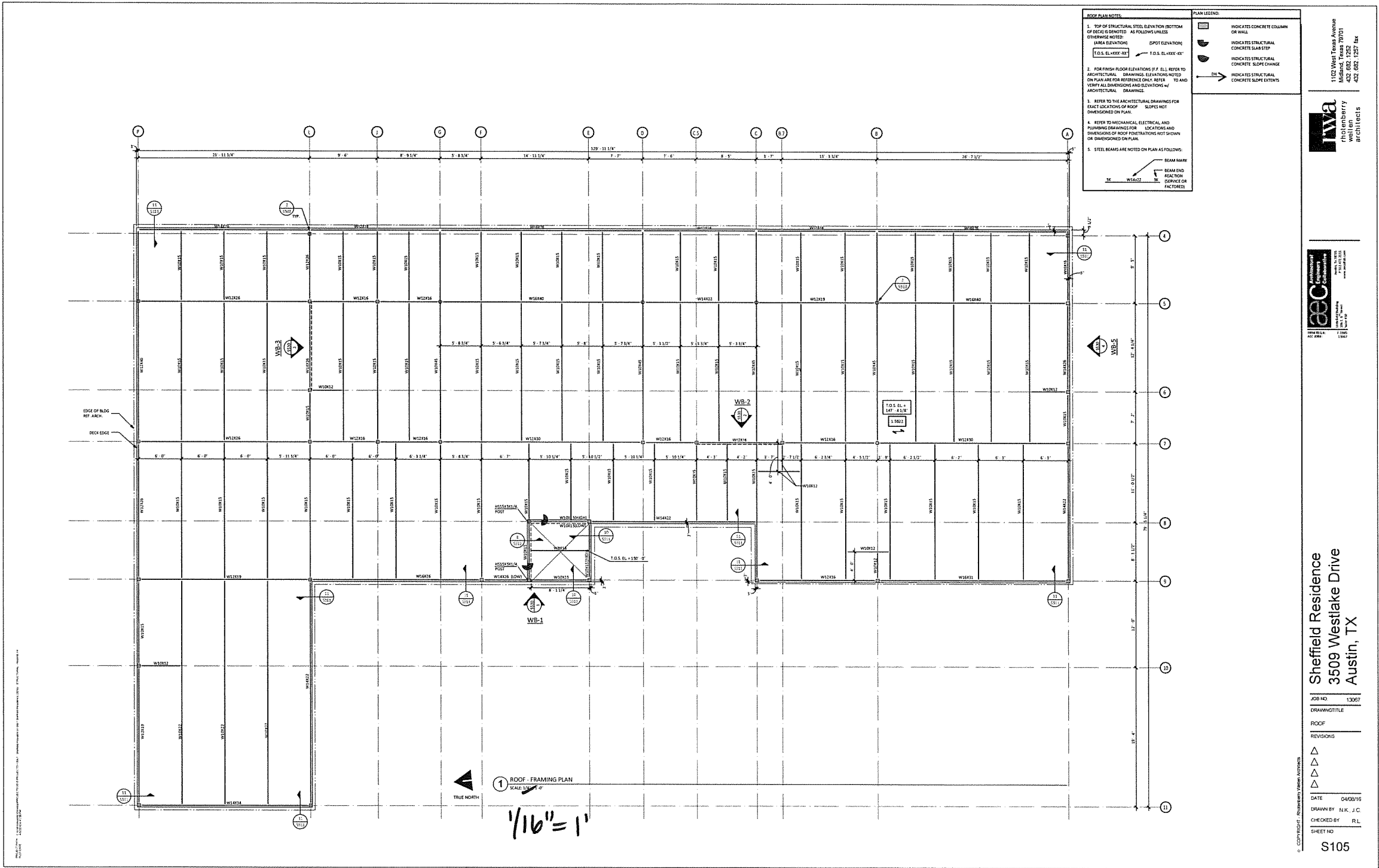
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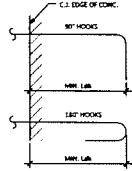


DOWEL SCHEDULE				
NAME		SIZE	A	B
DOWL A		#4	2'-4"	3'-0"
DOWL B		#4	2'-4"	3'-0"
DOWL C		#4	2'-4"	3'-0"
DOWL D		#4	2'-4"	3'-0"
DOWL E		#4	2'-4"	3'-0"

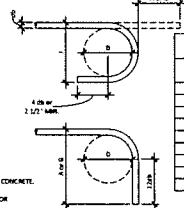
NOTES:
1. SCHEDULED DOWELS ARE MARKED "DOWL" ON THE SECTIONS AND DETAILS.
2. DOWEL SPACING TO BE SAME AS SLAB OR WALL REINFORCEMENT, UNLESS OTHERWISE NOTED ON DETAILS.

REINFORCEMENT SPIKE LENGTH SCHEDULE			
CLASS	14"	16"	18"
BAR SET			
#3	3'-2"	3'-2"	3'-2"
#4	3'-2"	3'-2"	3'-2"
#5	3'-2"	3'-2"	3'-2"
#6	3'-2"	3'-2"	3'-2"
#7	3'-2"	3'-2"	3'-2"
#8	3'-2"	3'-2"	3'-2"
#9	3'-2"	3'-2"	3'-2"
#10	3'-2"	3'-2"	3'-2"
#11	3'-2"	3'-2"	3'-2"

NOTES:
1. WHERE SPIKE TYPE IS NOT INDICATED, USE CLASS "B" SPIKE.
2. LAY LENGTHS SETTING ABOVE AND UNDER THE FOLLOWING CONDITIONS:
A. BEAM & COLUMN BARS ARE SPACED AT LEAST 3 BAR DIAMETERS O.C. WITH CLEAR COVER NOT LESS THAN 3 BAR DIAMETERS.
B. WALL & SLAB BARS ARE SPACED AT LEAST 3 BAR DIA. O.C.
C. FOR UNCLIPPED AND TIE-CONTINUED BARS, MINIMUM REINFORCEMENT.
D. FOR REINFORCEMENT THAT CONFORMS TO DEFORMED NEW BUILT STEEL BARS, IN ACCORDANCE TO ASTM A615, SEE 30.
3. WHERE CLEAR COVER OR CLEAR SPACING FOR MAIN REINFORCING IS LESS THAN 3 BAR DIAMETERS, INCREASE SPACING TO SHOWN BY ABOVE SPACING LONGITUDINALLY. BARS OF 3 BAR DIAMETERS TO CLEAR COVER OR SPACING. TABULATIONS BY 1/2".
4. FOR HOOKS, TOP BARS W/ 1/2" OF CONCRETE CAST BELOW, MULTIPLY TABULATIONS BY 1/2".
5. WHERE A LARGER BAR LAPS A SMALLER BAR, THE SMALLER SCHEDULED LAP LENGTH APPLIES O.C.
6. REFER TO "CONCRETE REINFORCING" SECTION OF THE GENERAL NOTES FOR FURTHER INFORMATION.



HOOK DEVELOPMENT LENGTH SCHEDULE	
BAR SIZE	DEVELOPMENT LENGTH
#3	12"
#4	12"
#5	12"
#6	12"
#7	12"
#8	12"
#9	12"
#10	12"
#11	12"

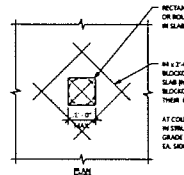


STANDARD HOOK SCHEDULE	
BAR SIZE	HOOK
#3	12"
#4	12"
#5	12"
#6	12"
#7	12"
#8	12"
#9	12"
#10	12"
#11	12"

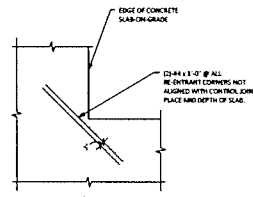
1 STANDARD DOWEL SCHEDULE
NO SCALE

2 LAP SPIKE SCHEDULE
NO SCALE

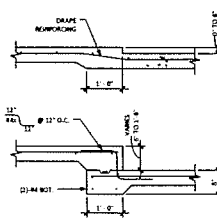
3 STANDARD HOOK SCHEDULE
NO SCALE



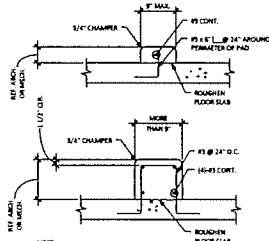
4 TYPICAL DETAIL -
ADDITIONAL REINFORCING AT
BLOCKOUT IN SLAB-ON-GRADE
NO SCALE



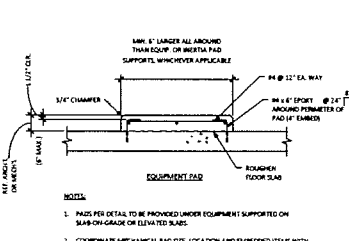
5 TYPICAL DETAIL -
SLAB-ON-GRADE RE-ENTRANT CORNER REINF.
NO SCALE



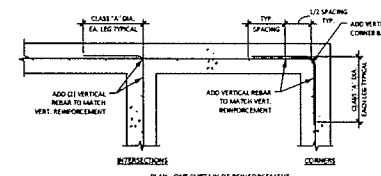
6 TYPICAL DETAIL -
DROP IN SLAB-ON-GRADE
NO SCALE



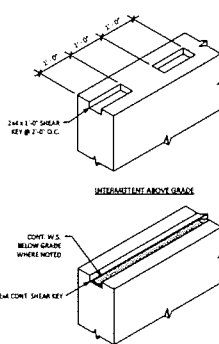
7 TYPICAL DETAIL -
SLAB-ON-GRADE OR STRUCTURAL SLAB MECHANICAL CURB
NO SCALE



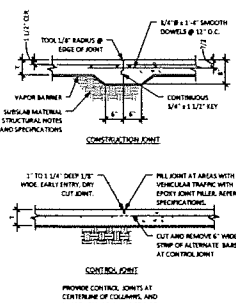
8 TYPICAL DETAIL -
MECHANICAL PAD
NO SCALE



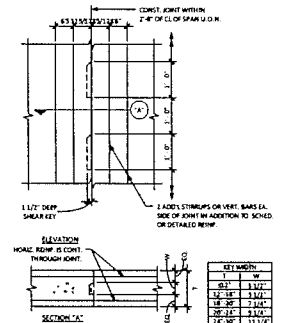
9 TYPICAL DETAIL -
WALL OR GRADE BEAM REINFORCING
NO SCALE



10 TYPICAL DETAIL -
GRADE BEAM SHEAR KEY AT HORIZONTAL JOINT
NO SCALE



11 TYPICAL DETAIL -
SLAB-ON-GRADE
NO SCALE

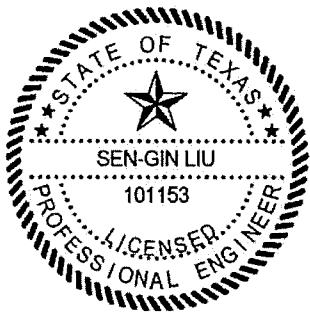


12 TYPICAL DETAIL -
DEEP GRADE BEAM AND WALL VERTICAL CONSTRUCTION JOINT
NO SCALE

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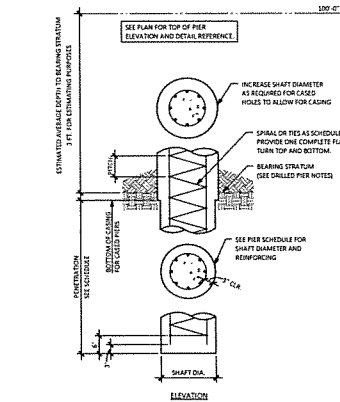
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04.08.2016

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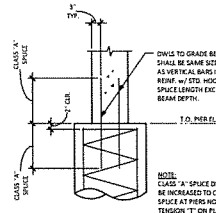
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CONCRETE TYPICAL
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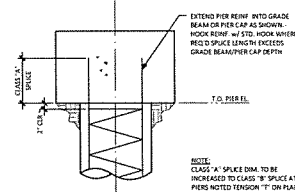
PIER SCHEDULE				
MARK	SHAFT DIAMETER	VERTICAL REINFORCING	SPIRALS	Penetration
P1	18"	(SEE #7's)	#3 @ 12" O.C.	18" - 0"
P2	24"	(SEE #7's)	#3 @ 12" O.C.	18" - 0"

- NOTE:
1. VERT. REINF. SHALL BE CLASS "A" SPLICED, TYP.
 2. VERT. REINF. FOR TENSION "T" PIERS SHALL BE CLASS "B" SPLICED.
 3. ROUND TIES THRU, (O) MAY BE USED IN LGS. OF SPIRALS. TIE LAP SHALL BE 12" MINIMUM.

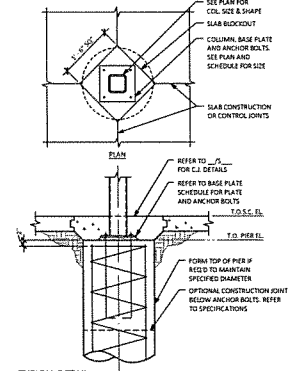
1 PIER DETAIL AND SCHEDULE
NO SCALE



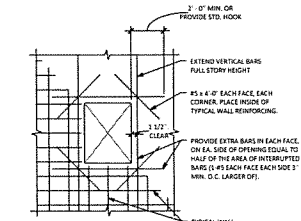
2 TYPICAL DETAIL
NARROW BEAM OR WALL BEARING ON WIDER PIER
NO SCALE



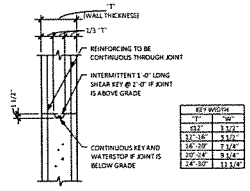
3 TYPICAL DETAIL
WIDE GRADE BEAM OR PIER CAP BEARING ON NARROW PIER
NO SCALE



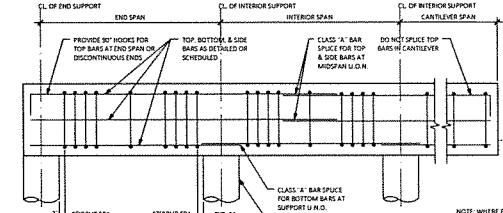
4 TYPICAL DETAIL
TOP OF PIER
NO SCALE



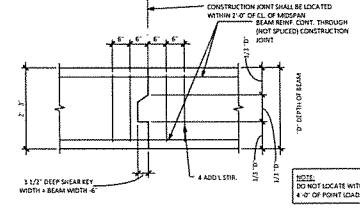
5 TYPICAL DETAIL
REINF. AT CONCRETE WALL OPENING
NO SCALE



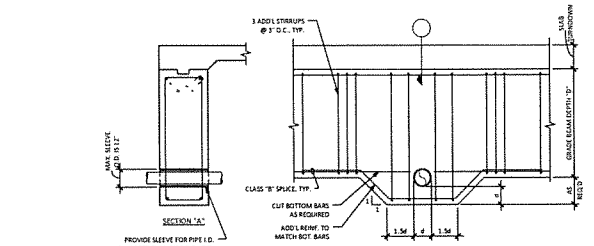
6 TYPICAL DETAIL
HORIZONTAL CONSTRUCTION JOINT IN CONCRETE WALL
NO SCALE



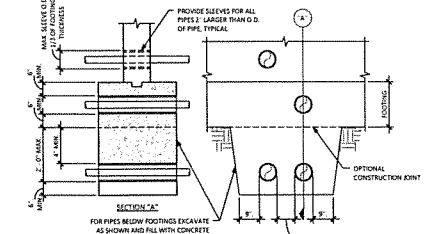
7 TYPICAL DETAIL
GRADE BEAM REINFORCING
NO SCALE



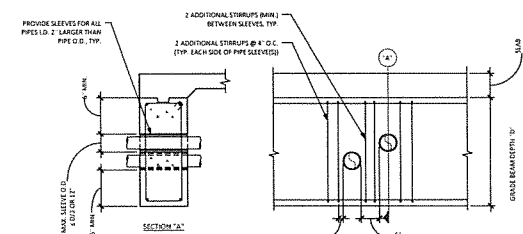
8 TYPICAL DETAIL
BEAM CONSTRUCTION JOINT
NO SCALE



9 TYPICAL DETAIL
HORIZONTAL PENETRATIONS THROUGH BOTTOM OF GRADE BEAM
NO SCALE



10 TYPICAL DETAIL
HORIZONTAL PIPE PENETRATIONS THROUGH AND/OR BELOW FOOTINGS
NO SCALE

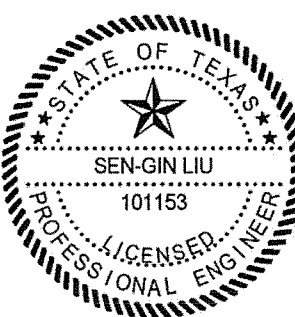


11 TYPICAL DETAIL
HORIZONTAL PIPE PENETRATIONS THROUGH GRADE BEAM
NO SCALE

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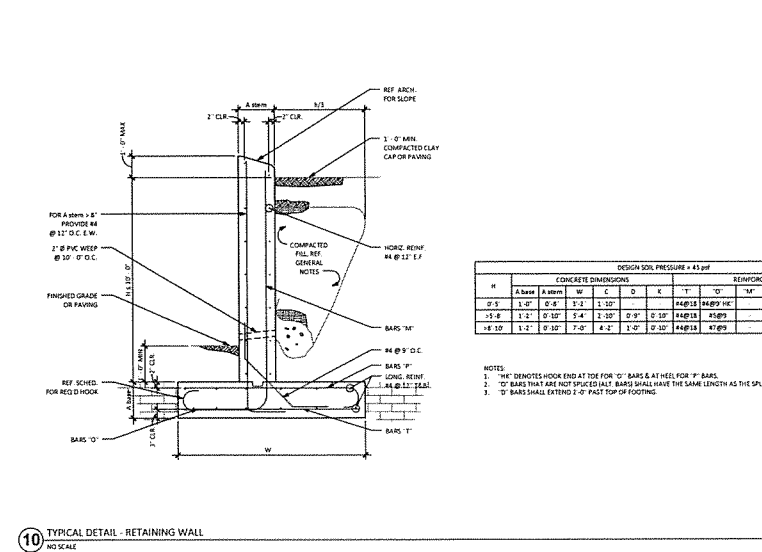
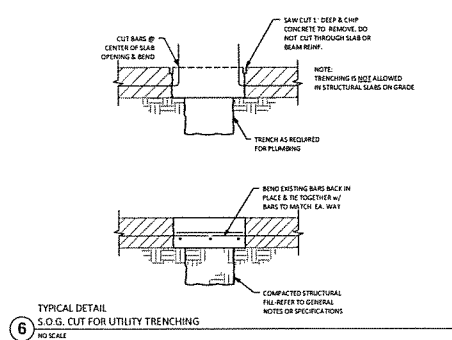
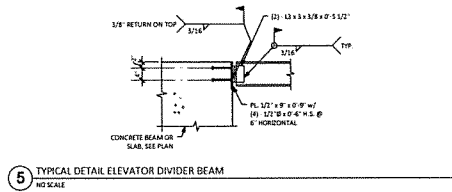
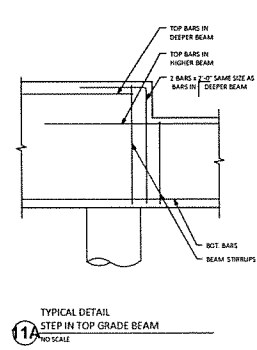
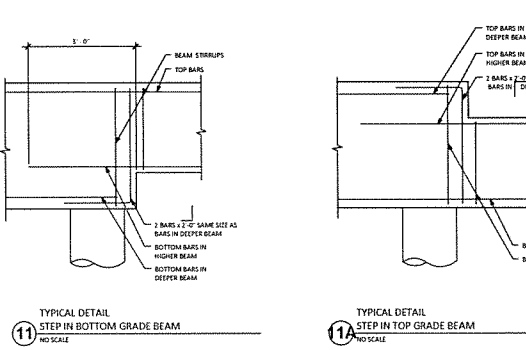
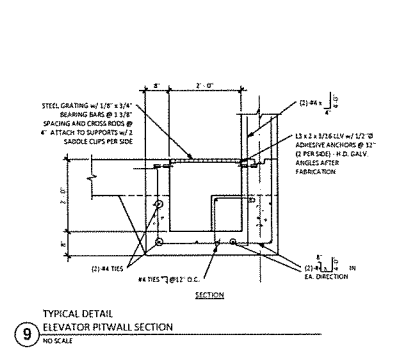
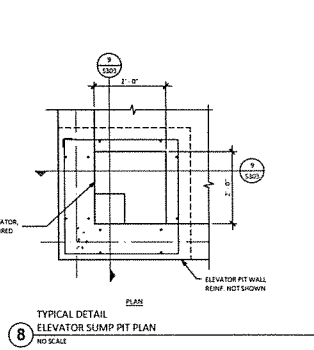
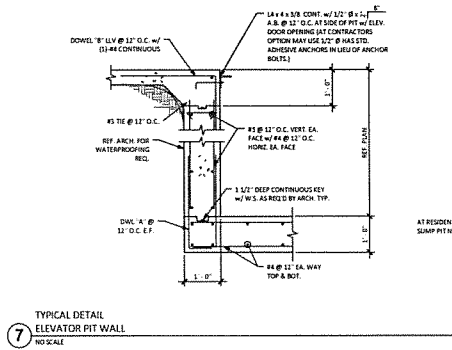
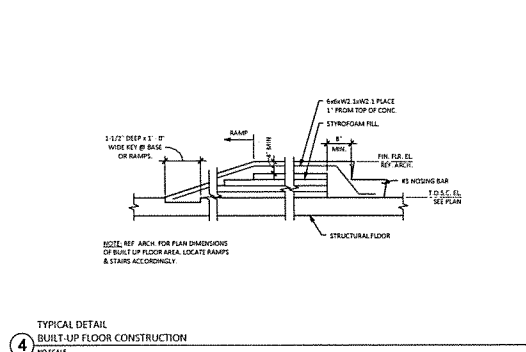
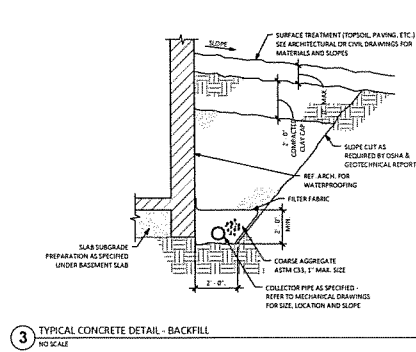
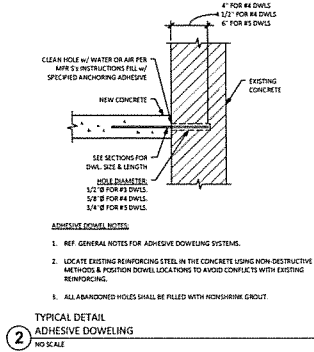
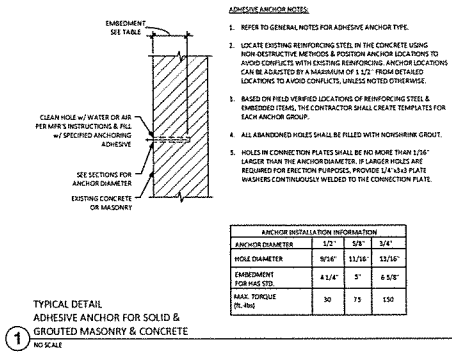
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04.08.2016

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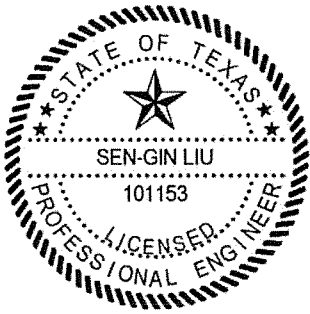
DESIGN SOIL PRESSURE = 4.5 psf												
H	CONCRETE DIMENSIONS					REINFORCING BARS						
	A	B	C	D	E	F	G	H	I	J	K	L
0'-0"	1'-0"	0'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"
1'-0"	1'-0"	0'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"
2'-0"	1'-0"	0'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"

NOTES:
1. "H" DENOTES HOOK END AT TOP FOR "O" BARS & AT HEEL FOR "F" BARS.
2. "O" BARS THAT ARE NOT SPICED HAVE THE SAME LENGTH AS THE SPICED BARS.
3. "F" BARS SHALL EXTEND 2'-0" FROM TOP OF FOOTING.

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rwa
Richard W. Anderson
Professional Engineer
101153

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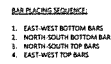


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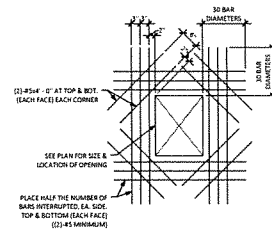
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S303



1 TWO-WAY SLAB BAR PLACEMENT
NO SCALE



2 SLAB OPENING REBAR PLACEMENT
IN MILD REINF. TWO-WAY SLAB
NO SCALE

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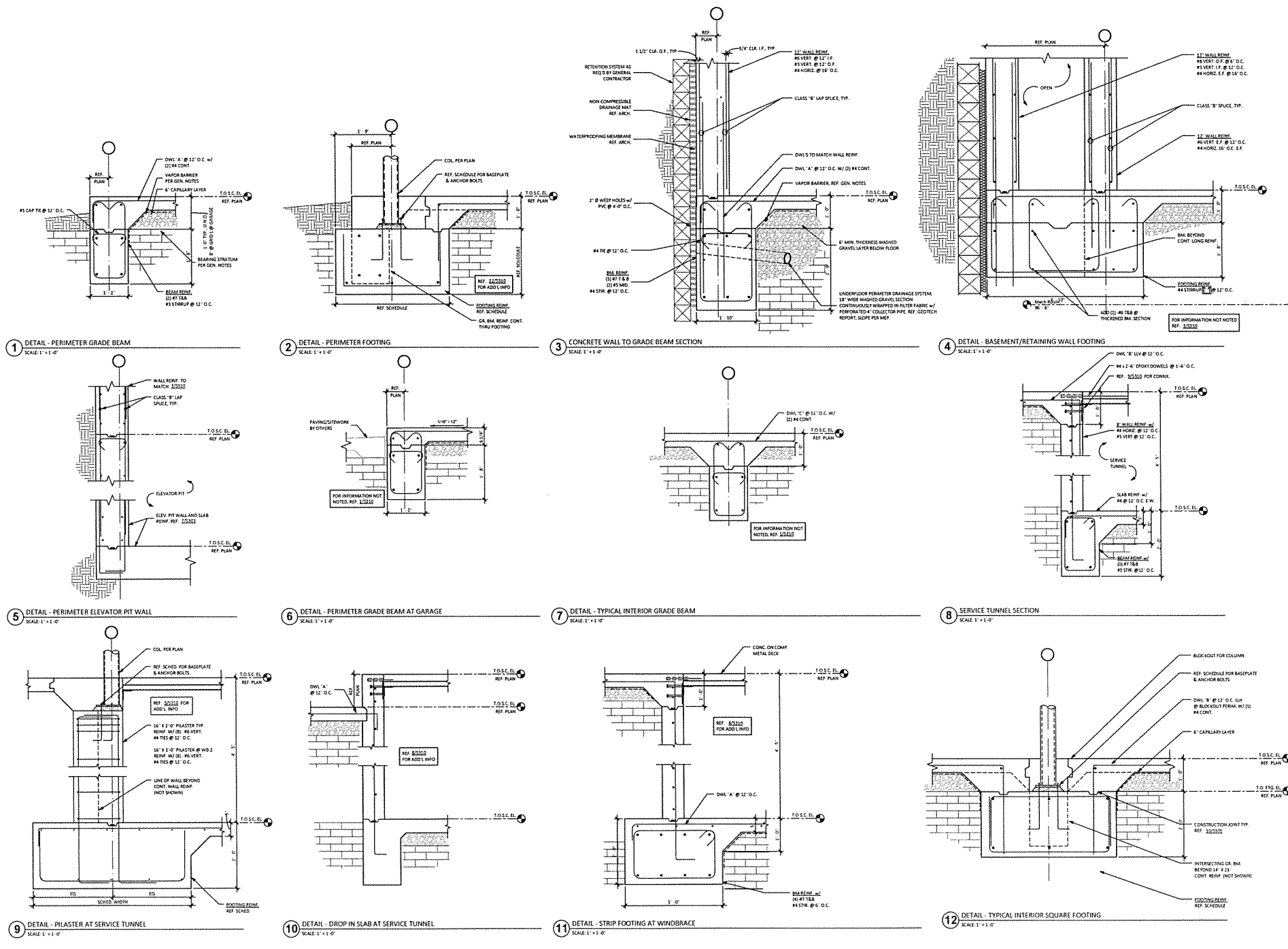
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Ann. Jui Li
04.08.2016



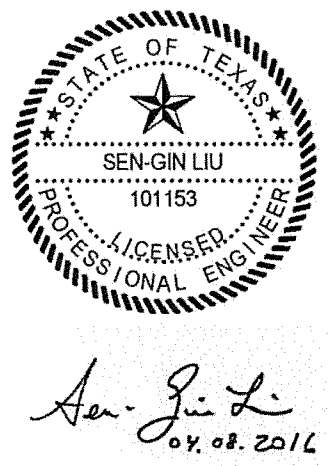
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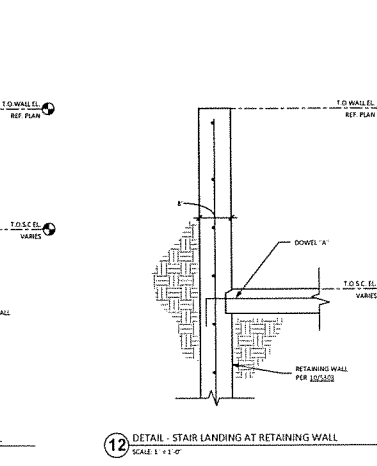
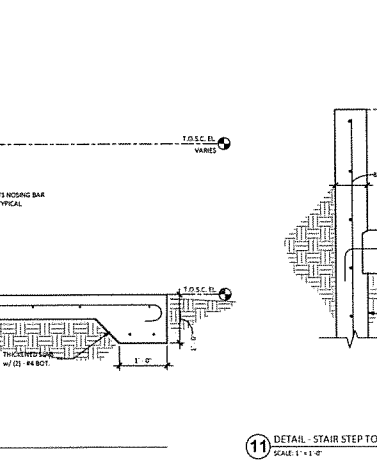
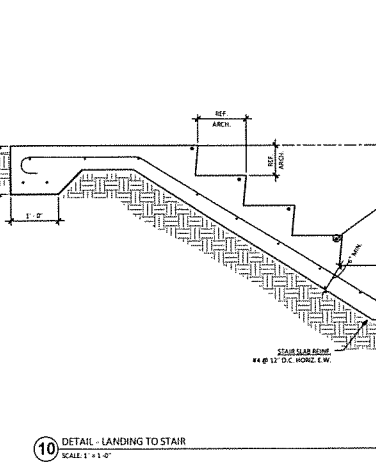
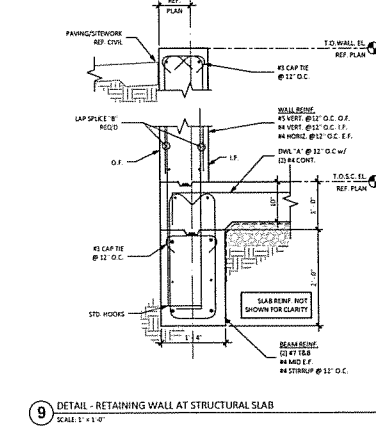
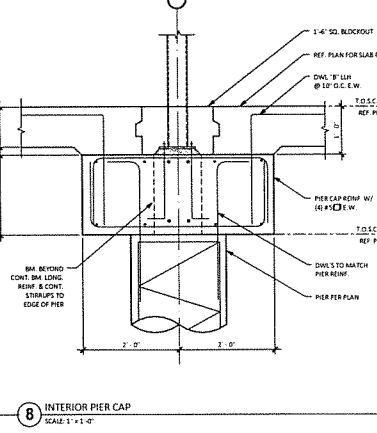
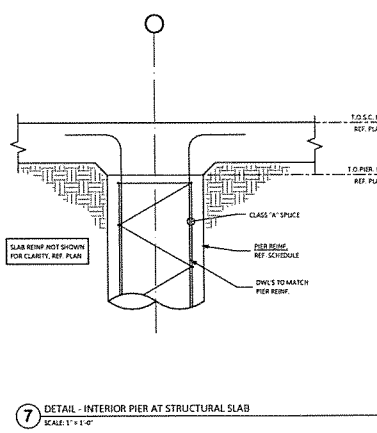
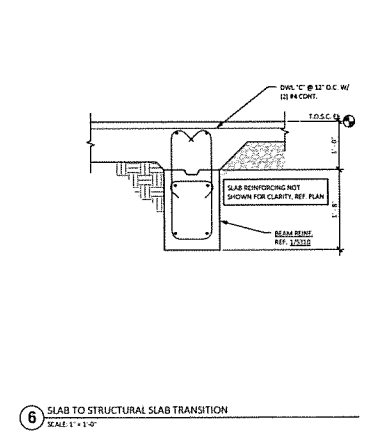
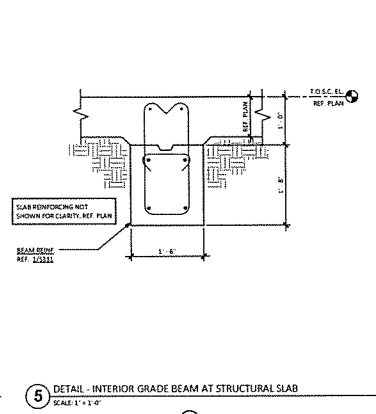
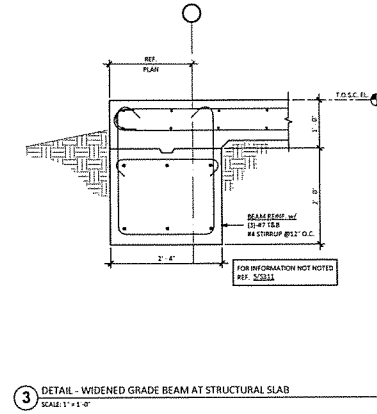
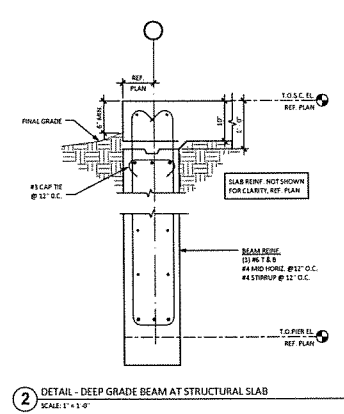
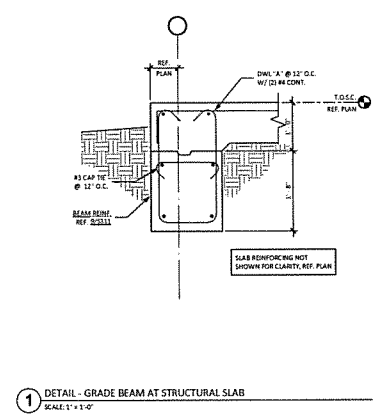
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1" = 20'

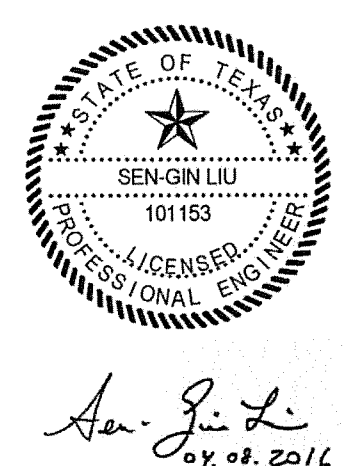


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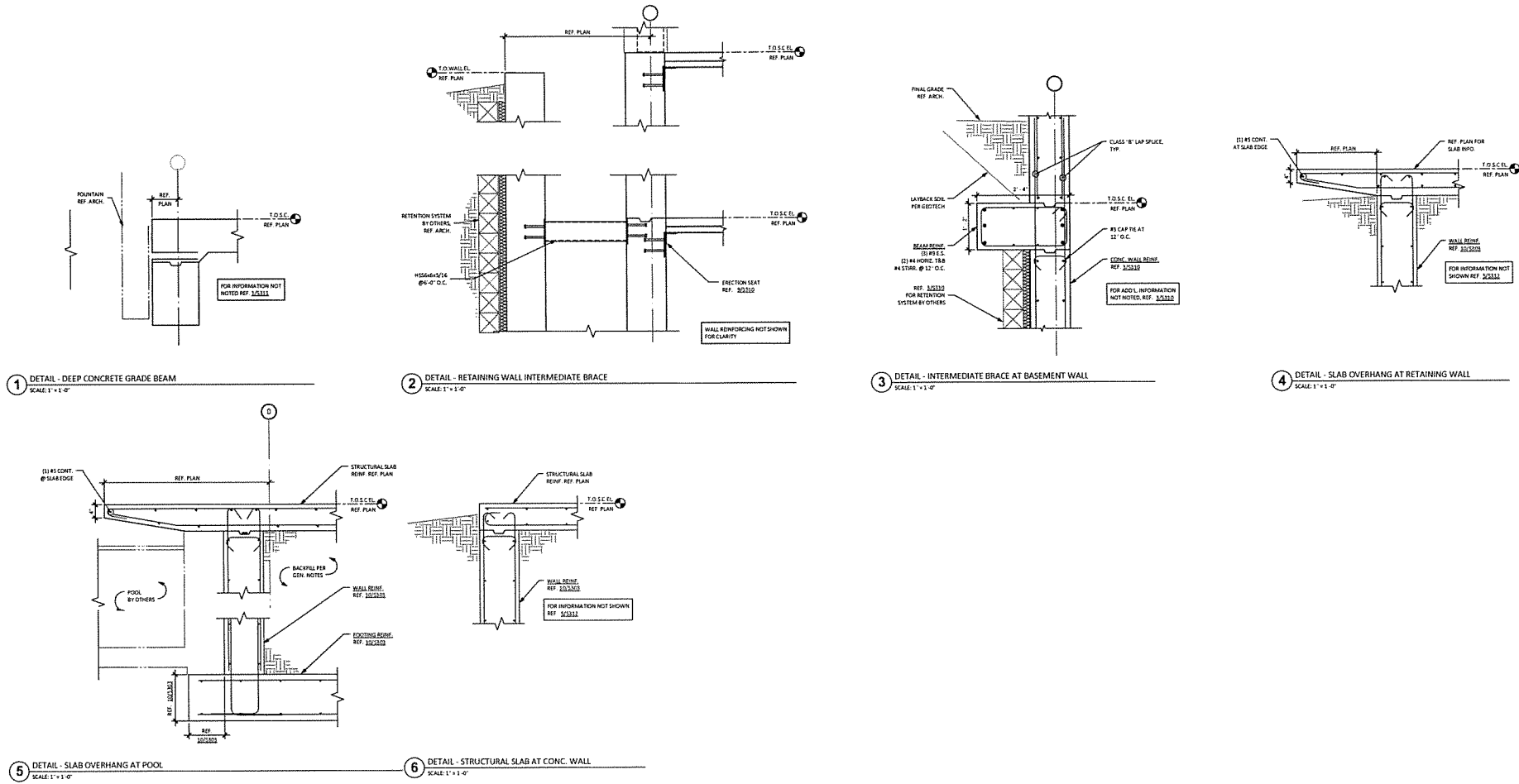
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1"=30'

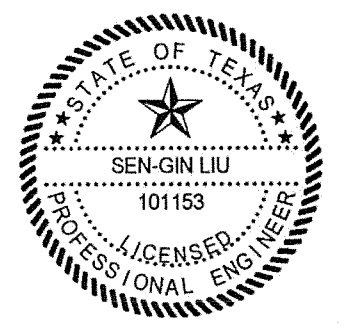


1" = 30'

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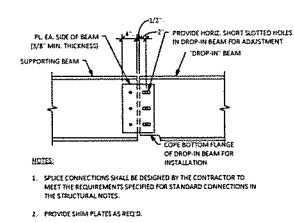


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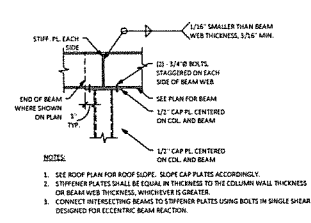
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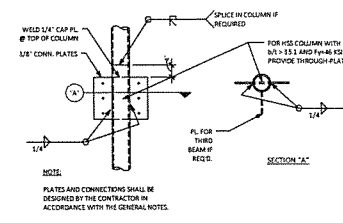
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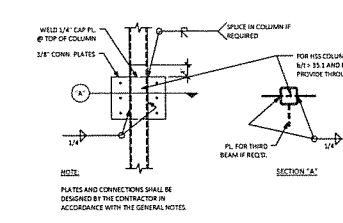
1 TYPICAL DETAIL
BOLTED BEAM SPICE
NO SCALE



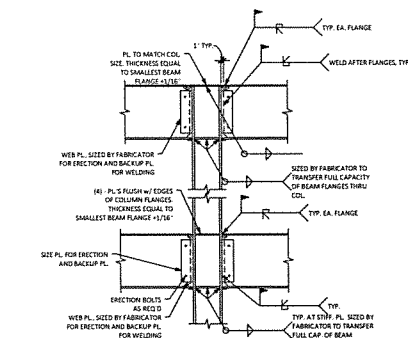
2 TYPICAL DETAIL
CAP PLATE-BOLTED CONNECTION
NO SCALE



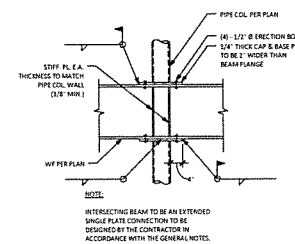
3 TYPICAL DETAIL
PIPE COLUMN CONNECTION FOR WEB CONNECTIONS TO BEAM
NO SCALE



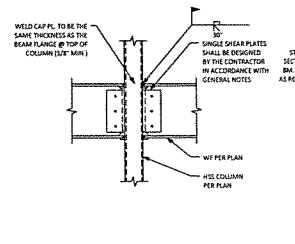
4 TYPICAL DETAIL
TUBE COLUMN CONNECTION FOR WEB CONNECTIONS TO BEAM
NO SCALE



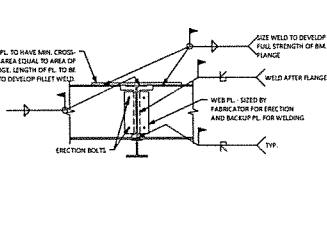
5 TYPICAL DETAIL
BEAM TO TOP OF COLUMN MOMENT CONNECTION
NO SCALE



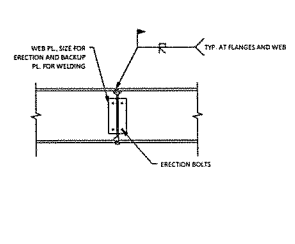
6 TYPICAL DETAIL
HSS COLUMN SPLICED TO CONTINUOUS BEAM
NO SCALE



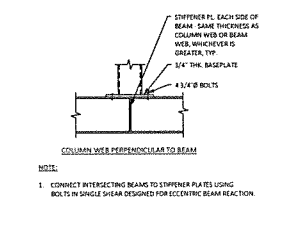
7 TYPICAL DETAIL
MOMENT CONNECTION THROUGH TUBE COLUMN
NO SCALE



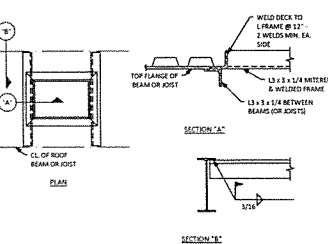
8 TYPICAL DETAIL
MOMENT CONNECTION 1
NO SCALE



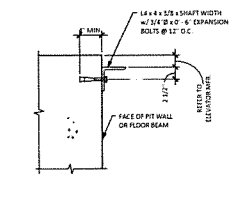
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MOMENT CONNECTION
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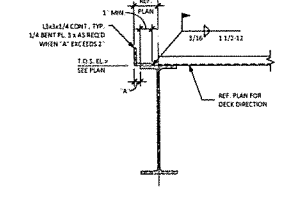
10 TYPICAL DETAIL
COLUMN SUPPORTED ON BEAM CONNECTION
NO SCALE



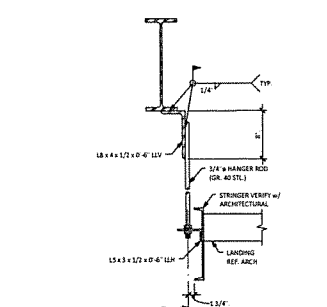
11 TYPICAL DETAIL
ROOF OPENING
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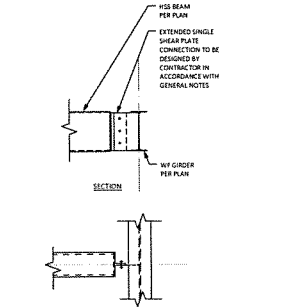
12 TYPICAL DETAIL
ELEVATOR DOOR SILL
NO SCALE



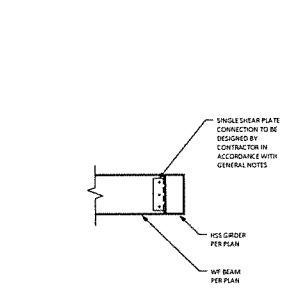
13 TYPICAL DETAIL
EXTERIOR BEAM
NO SCALE



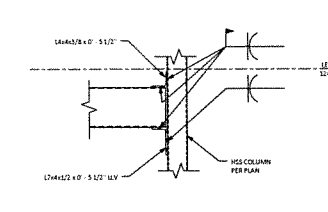
14 TYPICAL DETAIL
METAL STAIR SUPPORT
NO SCALE



15 TYPICAL DETAIL
HSS BEAM CONNECTION TO WIDE FLANGE
NO SCALE



16 TYPICAL DETAIL
WIDE FLANGE BM. TO HSS CONNECTION
NO SCALE



17 TYPICAL DETAIL
HSS BEAM CONNECTION TO HSS COLUMN
NO SCALE

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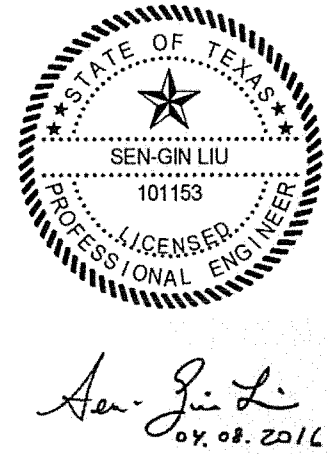
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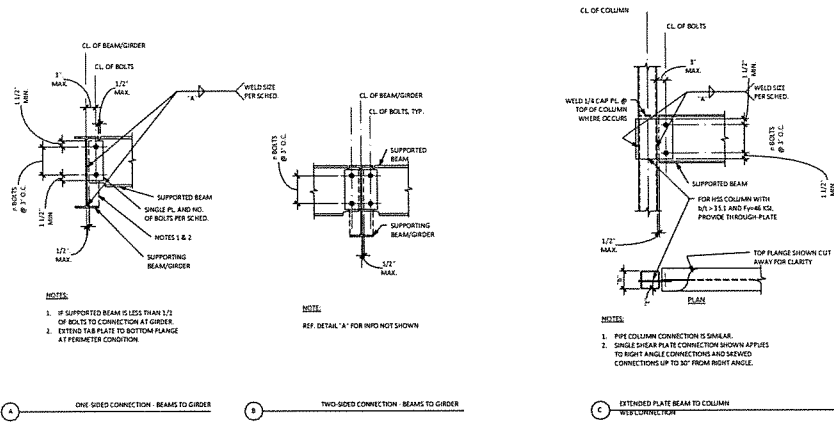
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DATE 04/06/15
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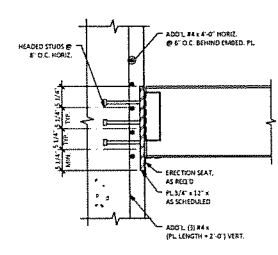




STANDARD SINGLE PLATE CONNECTION					
BEAM SIZE	NO. OF ROWS OF BOLTS	BOLT DIA. (IN)	PLATE THICK. (IN)	WELD SIZE (IN)	MAX. BEAM REACTION (KIPS)
W8	2	3/4"	1/4"	1/4"	12
W10	2	3/4"	1/4"	1/4"	16
W12	3	3/4"	1/4"	1/4"	24
W14	3	3/4"	1/4"	1/4"	30
W16	4	3/4"	1/4"	1/4"	40
W18	5	3/4"	1/4"	1/4"	50
W21	6	3/4"	1/4"	1/4"	73
W24	7	3/4"	1/4"	1/4"	83
W27	8	3/4"	1/4"	1/4"	97
W30	8	3/4"	1/4"	1/4"	97
W31	8	3/4"	1/4"	1/4"	97
W36	10	3/4"	1/4"	1/4"	140
W40	10	3/4"	1/4"	1/4"	140
W44	10	3/4"	1/4"	1/4"	140

- NOTES:
- ALL OTHER CONNECTIONS DEVIATING FROM TYPICAL CONNECTIONS SHALL BE DESIGNED AND DETAILLED BY A PROFESSIONAL ENGINEER WORKING UNDER THE GUIDANCE OF THE CONTRACTOR. REF. GENERAL NOTES UNDER "STRUCTURAL STEEL CONNECTIONS."
 - NOTED REACTIONS ARE FOR SERVICE LOADS.
 - BOLTS ARE ASSUMED WITH STANDARD HOLES.
 - SCHEDULED SHEAR PLATE CONNECTIONS APPLY TO RIGHT ANGLE CONNECTIONS AND SKEWED CONNECTIONS UP TO 30° FROM RIGHT ANGLE.
 - BEAM CONNECTIONS ARE "STANDARD" UNLESS OTHERWISE NOTED ON PLAN.
 - WORKSPACES ARE ON CENTERLINES OF BEAMS AND COLUMNS, U.O.B.
 - WELD CAPACITY BASED ON E60 = 70 KSI.
 - CONTRACTOR RESPONSIBLE FOR MEETING ALL U.S.A. REQUIREMENTS.

1 TYPICAL DETAIL
SCHEMATIC SINGLE-PLATE FRAMING CONNECTIONS
NO SCALE



MAX. ALLOWABLE END REACTION SERVICE LOADS (KIPS)	NUMBER OF STUDS	PLATE LENGTH (NOTE 4)
20 K	4	1'-2"
40 K	6	1'-7"
55 K	8	2'-0"
75 K	10	2'-5"
95 K	12	2'-10"
115 K	14	3'-3"
140 K	16	3'-8"
160 K	18	4'-2"

- NOTES:
- PROVIDE EMBED PLATE TO EQUAL OR EXCEED BEAM REACTION AS SPECIFIED FOR BEAM CONNECTION DESIGN.
 - DOUBLE ANGLE CONNECTION SHALL BE DESIGNED AND DETAILLED BY A PROFESSIONAL ENGINEER WORKING UNDER THE GUIDANCE OF THE CONTRACTOR. REF. GENERAL NOTES UNDER "STRUCTURAL STEEL CONNECTIONS."
 - HEADERS SHALL BE 7/8" W x 6" D.
 - PROVIDE EXTRA PLATE LENGTH AS REQ'D FOR ERECTION SEAT AT CONTRACTOR OPTION.
 - CONTRACTOR RESPONSIBLE FOR COORDINATING CONFLICTS WITH ARCH'L.

5 TYPICAL DETAIL
STEEL BEAM TO CONCRETE CONNECTION
NO SCALE

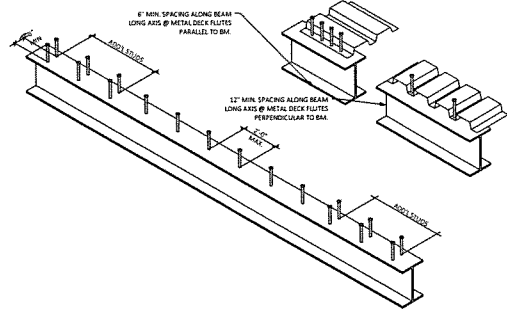
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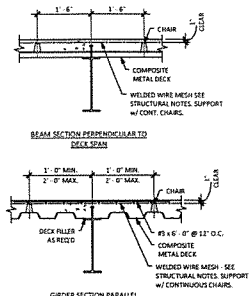
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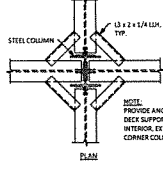


1 TYPICAL DETAIL
SHEAR STUD PLACEMENT DIAGRAM
NO SCALE

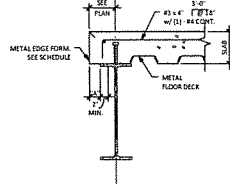
- NOTES:
1. DETERMINE WHERE ALL STUDS ARE TO BE PLACED BEFORE WELDING TO BEAMS. SEE DETAIL FOR STUD SPACING AND EDGE DISTANCE REQUIREMENTS.
 2. FOR UNIFORMLY DISTRIBUTED STUDS: PLACE HALF OF THE TOTAL NUMBER OF STUDS EACH SIDE OF THE BEAM AND SPAN FOR NON-UNIFORMLY DISTRIBUTED STUDS: PLACE NUMBER STUDS BETWEEN BEAMS AS NOTED ON PLAN.
 3. AT BEAMS PERPENDICULAR TO DECK FLUTES: STARTING FROM THE END SUPPORT PLACE SHEAR STUDS UNIFORMLY TOWARD THE MIDSPAN. MAX SPACING SHALL NOT EXCEED 24 INCHES. FOR REMAINING STUDS, BEGIN FROM EACH END AND PLACE STUDS AT REMAINING FLUTES WITHOUT STUDS. WHERE REQUIRED NUMBER OF STUDS EXCEEDS SINGLE STUDS PER FLUTE, PLACE EXTRA STUDS IN PAIRS (L2) EACH SIDE OF BEAM WEB, STARTING AT EACH END OF BEAM. WHERE REQUIRED NUMBER OF STUDS EXCEEDS PAIR, PLACE EXTRA STUDS IN TRIPS (ONE AT CENTER, PLUS 3\"/>



2 TYPICAL DETAIL
COMPOSITE DECK
NO SCALE



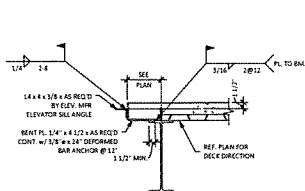
3 TYPICAL DETAIL
DECK SUPPORT AT WF COLUMN
NO SCALE



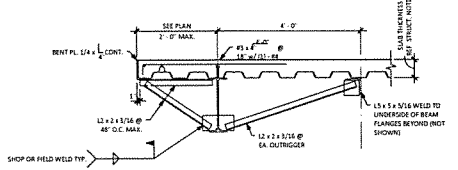
4 TYPICAL DETAIL
COMPOSITE SLAB EDGE CONDITION
NO SCALE

MAXIMUM SLAB OVERHANG "A"			
EDGE FORM GAUGE	SLAB THICKNESS		
18 GA.	4 1/2"	5 1/2"	6 1/2"
14 GA.	6"	6"	7"
12 GA.	7"	8"	8"
10 GA.	10"	11"	10"

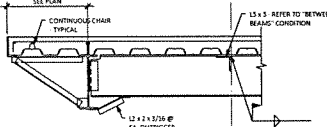
"A" DIMENSION "A" EXCEEDS TABLE MAXIMUMS. PROVIDE ANGLE SUPPORT FOR DETAIL SCALE. PROVIDE 1/2" RETURN UP ON 18 GA. EDGE FORM.



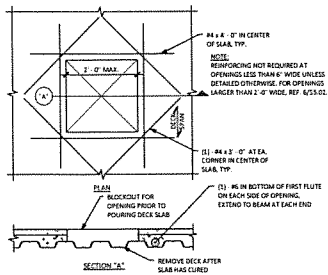
5 TYPICAL DETAIL
EDGE CONDITION AT STAIR STRINGERS AND ELEVATOR SILL
NO SCALE



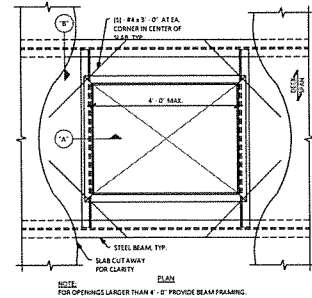
6 TYPICAL DETAIL
CHANGE OF DECK DIRECTION OVER BEAM
NO SCALE



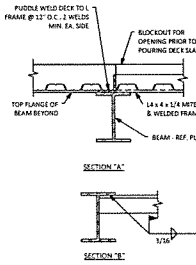
7 TYPICAL DETAIL
DECK OVERHANGING SPANREL BEAM
NO SCALE



8 TYPICAL DETAIL
SMALL MECHANICAL OPENING IN COMPOSITE SLAB
NO SCALE



9 TYPICAL DETAIL
LARGE MECHANICAL OPENING IN COMPOSITE SLAB
NO SCALE



1102 West Texas Avenue
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402.882.1231 fax

iwat
PHOTOGRAPHY
wallen
architects

aecc
ARCHITECTURAL
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CONSULTANTS
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04.08.2016

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Austin, TX

JOB NO 13067

DRAWING TITLE

COMPOSITE STEEL

TYPICAL DETAILS

REVISIONS

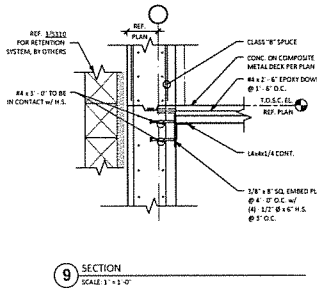
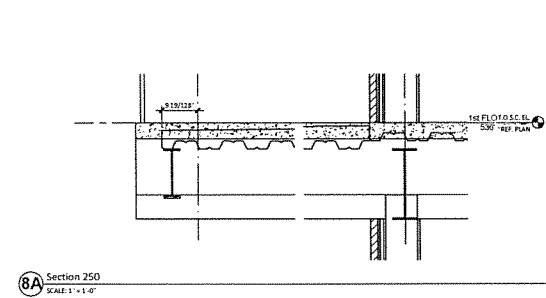
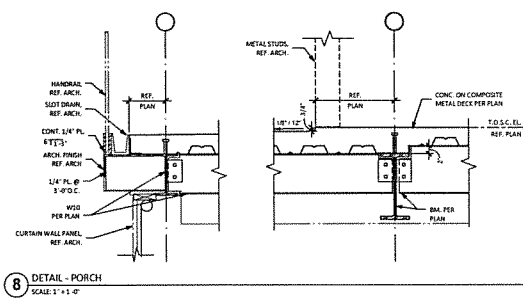
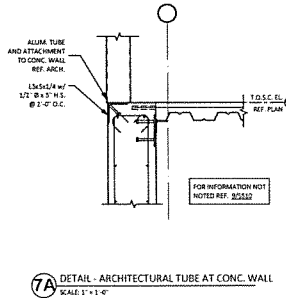
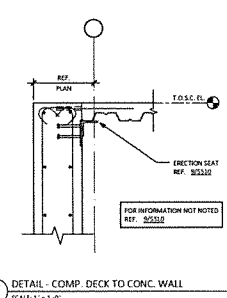
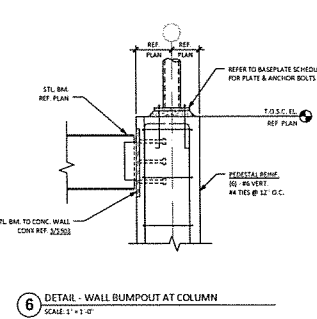
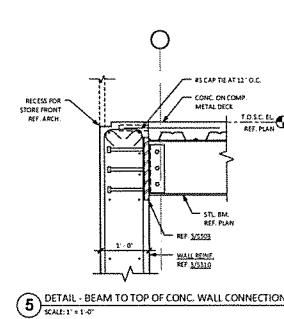
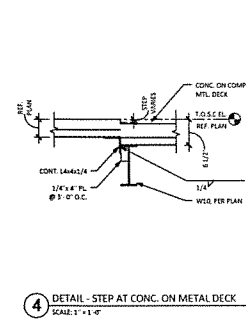
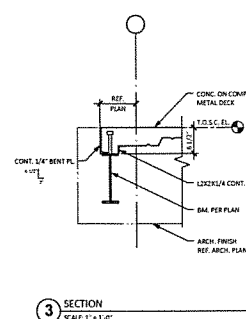
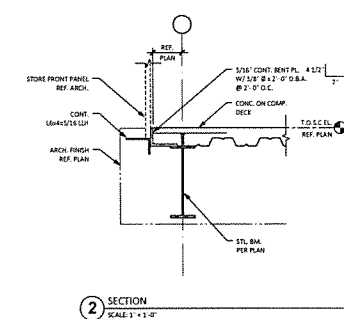
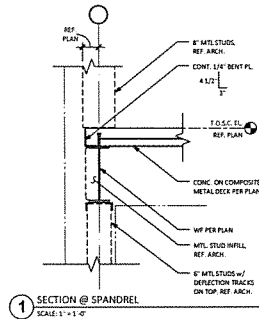
DATE 04/05/16

DRAWN BY N.K. J.C.

CHECKED BY R.L.

SHEET NO

S504

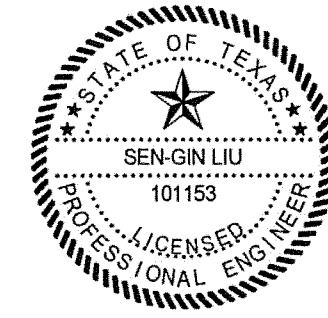


1" = 30'

1103 West Travis Avenue
Midland, Texas 79701
432.682.1252
432.682.1257 fax

rwa
RICHARD W. ANDERSON
ARCHITECTS

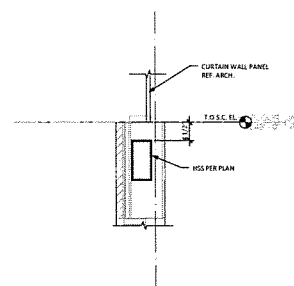
aec
ARCHITECTURAL ENGINEERS
CONSULTANTS



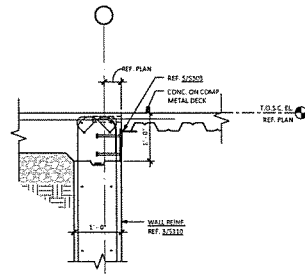
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04.08.2016

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3509 Westlake Drive
Austin, TX

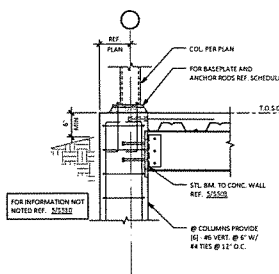
JOB NO. 13067
DRAWING TITLE
STEEL DETAILS
REVISIONS
DATE 04/08/16
DRAWN BY H.K.J.C.
CHECKED BY R.L.
SHEET NO.
S510



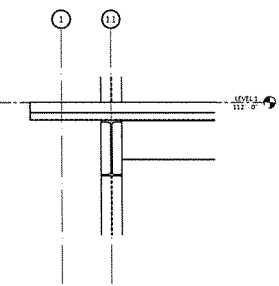
1 DETAIL - INTERMEDIATE CURTAIN WALL SUPPORT
SCALE: 1" = 1'-0"



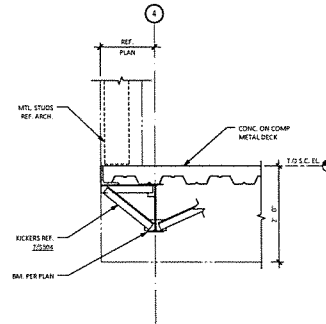
2 DETAIL - COMP. DECK TO CONC. WALL AND SLAB
SCALE: 1" = 1'-0"



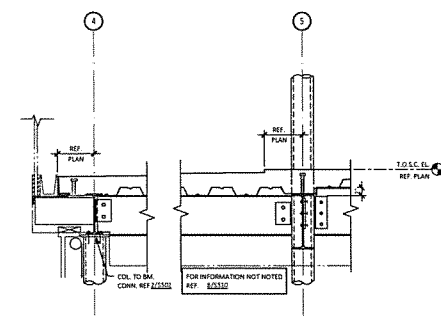
3 COLUMN BASEPLATE DETAIL
SCALE: 1" = 1'-0"



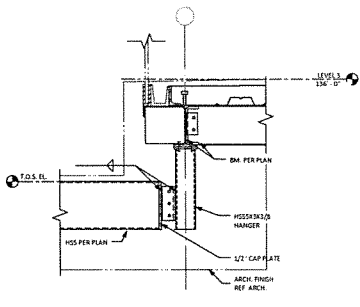
4 DETAIL - EDGE AT SECOND FLOOR PORCH
SCALE: 1" = 1'-0"



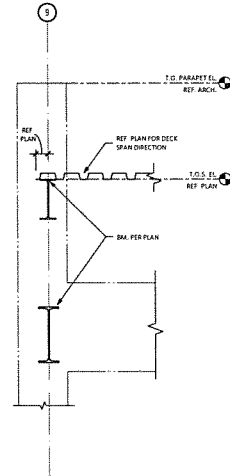
5 DETAIL - DECK EDGE AT LIVING ROOM
SCALE: 1" = 1'-0"



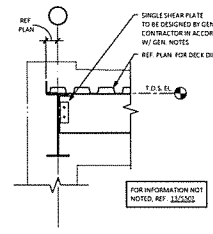
6 DETAIL - PORCH SECTION
SCALE: 1" = 1'-0"



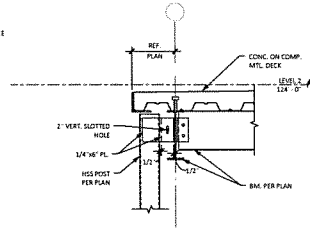
7 3RD FLOOR HANGER CONNECTION
SCALE: 1" = 1'-0"



8 DETAIL - PARAPET AT ELEVATOR
SCALE: 1" = 1'-0"



9 DETAIL - ROOF EDGE SECTION
SCALE: 1" = 1'-0"



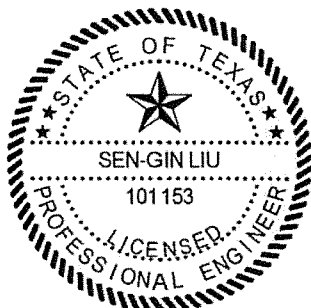
10 DETAIL - HSS POST TO BOTTOM OF BEAM CORN.
SCALE: 1" = 1'-0"

1" = 30'

1102 West Texas Avenue
Austin, TX 78701
432.682.1252
432.682.1257 fax

rwa
Robb
architects

aec
Architectural
Engineering
Consultants

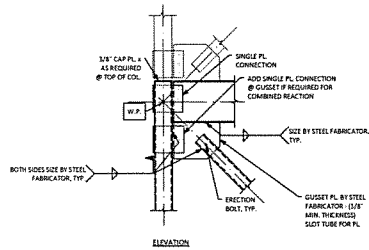


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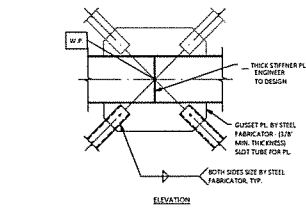
Sheffield Residence
3509 Westlake Drive
Austin, TX

JOB NO. 13057
DRAWING TITLE
STEEL DETAILS
REVISIONS
DATE 04/08/16
DRAWN BY N.K.J.C.
CHECKED BY RL
SHEET NO.

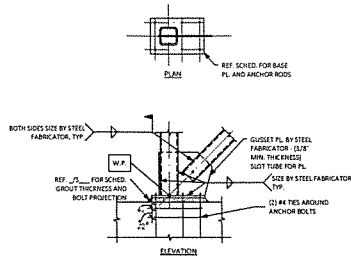
S511



1 TYPICAL DETAIL
WIDE FLANGE BEAM AND WIND
BRACE CONNECTION TO TUBE COLUMN
NO SCALE



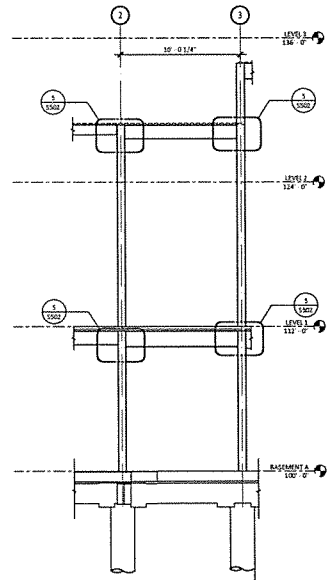
2 TYPICAL DETAIL
WIND BRACE CONNECTION TO BEAM
NO SCALE



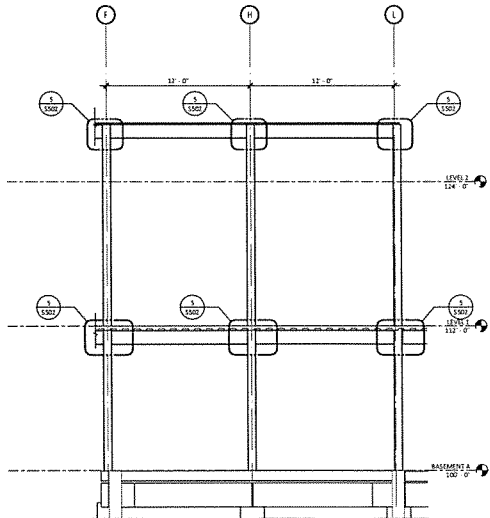
3 TYPICAL DETAIL
WIND BRACE CONNECTION TO TUBE COLUMN AND BASE PLATE
NO SCALE

WIND BRACE DESIGN NOTES

- Design connections for forces shown on wind brace elevations.
- If no forces are shown on wind brace elevations, design for the full tensile capacity of the brace according to the Manual of American Institute of Steel Construction, Thirteenth Edition.
- See notes for force & eccentricity of C.G. weld & C.G. force.
- At beam to column connections, size attachment to column for combined gravity load shear plus vertical shear component due to lateral load.
- Check for Whitmore section buckling, and block shear rupture of gusset plates.
- Calculations shall be performed and sealed by a Registered Professional Engineer & shall be submitted to the Architect per specifications.
- Design forces noted as V_u are factored forces in axial tension or compression. V_u are factored vertical shear forces.



4 ELEVATION - MOMENT FRAME
SCALE: 1/4" = 1'-0"

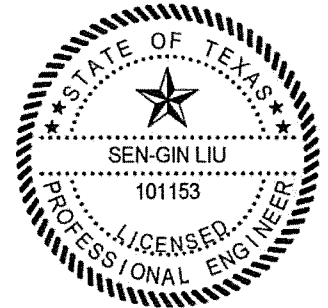


5 MOMENT FRAME - 2
SCALE: 1/4" = 1'-0"

1/16" = 1'

iwa
interior wall
architects
1102 West Texas Avenue
Midland, Texas 79701
432.682.1252
432.682.1257 fax

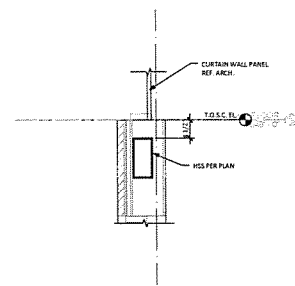
aecc
Architectural
Engineers
Consultants
1102 West Texas Avenue
Midland, Texas 79701
432.682.1252
432.682.1257 fax



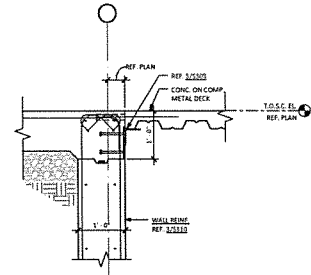
Sen-Gin Liu
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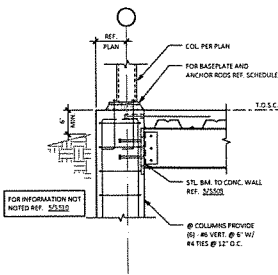
JOB NO. 13067
DRAWING TITLE
WIND BRACE HSS
TYPICAL DETAILS
REVISIONS
DATE 04/08/16
DRAWN BY H.K.J.C.
CHECKED BY R.L.
SHEET NO. S531



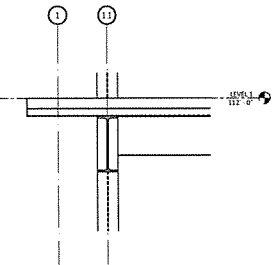
1 DETAIL - INTERMEDIATE CURTAIN WALL SUPPORT
SCALE: 1" = 1'-0"



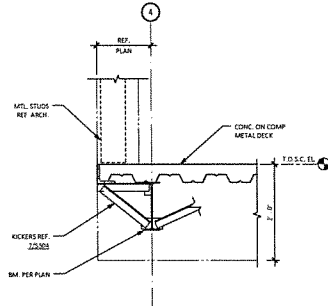
2 DETAIL - COMP. DECK TO CONC. WALL AND SLAB
SCALE: 1" = 1'-0"



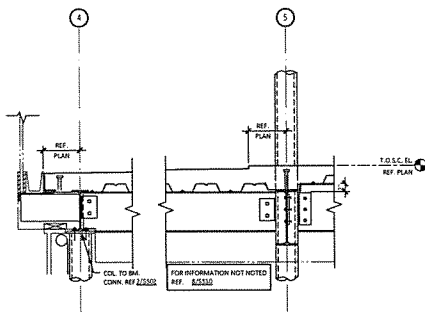
3 COLUMN BASEPLATE DETAIL
SCALE: 1" = 1'-0"



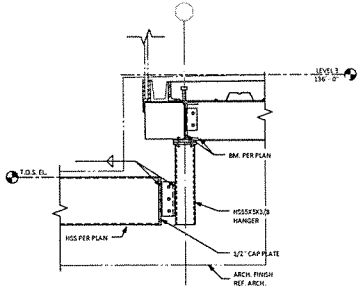
4 DETAIL - EDGE AT SECOND FLOOR PORCH
SCALE: 1" = 1'-0"



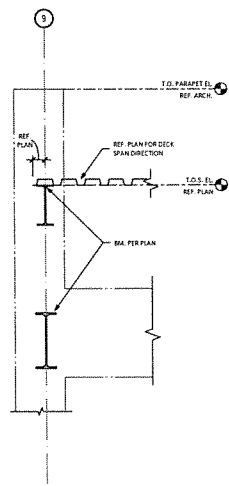
5 DETAIL - DECK EDGE AT LIVING ROOM
SCALE: 1" = 1'-0"



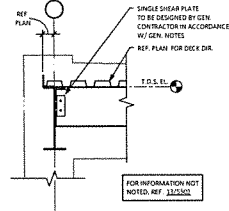
8 DETAIL - PORCH SECTION
SCALE: 1" = 1'-0"



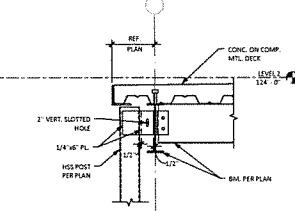
9 3RD FLOOR HANGER CONNECTION
SCALE: 1" = 1'-0"



10 DETAIL - PARAPET AT ELEVATOR
SCALE: 1" = 1'-0"



11 DETAIL - ROOF EDGE SECTION
SCALE: 1" = 1'-0"



12 DETAIL - HSS POST TO BOTTOM OF BEAM CONN.
SCALE: 1" = 1'-0"

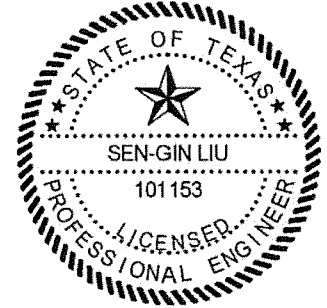
1" = 30'

rwa
RICHMOND
WATKINS
ARCHITECTS
1102 West Texas Avenue
Austin, TX 78701
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402.852.1257 fax

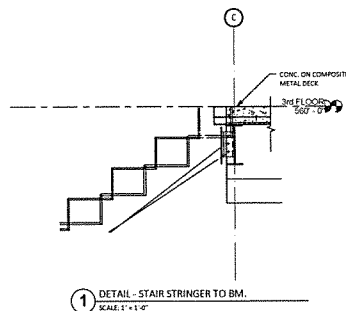
aec
ARCHITECTURAL
ENGINEERING
CONSULTANTS
10000 N. Mopac Expressway
Suite 1000
Austin, TX 78753
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512.452.1101 fax

Sheffield Residence
3509 Westlake Drive
Austin, TX

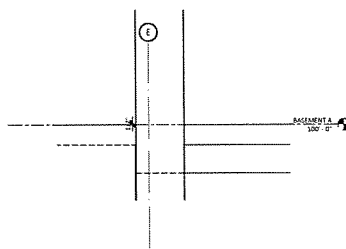
JOB NO. 13057
DRAWING TITLE
STEEL DETAILS
REVISIONS
DATE 04/03/16
DRAWN BY N.K., J.C.
CHECKED BY R.L.
SHEET NO.
S511



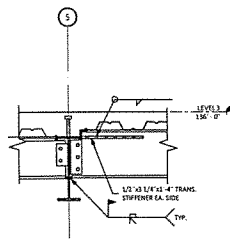
Sen-Gin Liu
04.08.2016



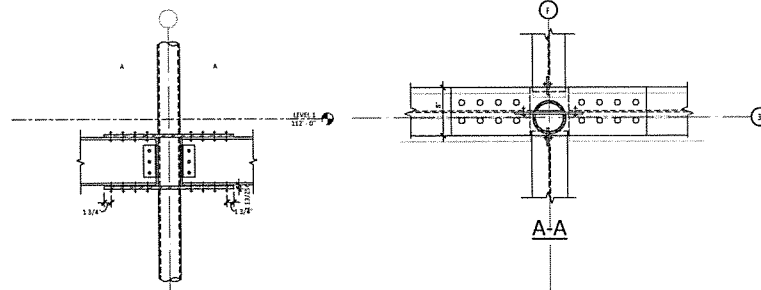
1 DETAIL - STAIR STRINGER TO BM.
SCALE: 1" = 1'-0"



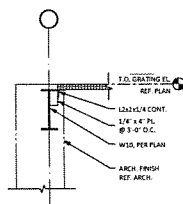
2 DETAIL - BASE PLATE AT STAIR STRINGER
SCALE: 1" = 1'-0"



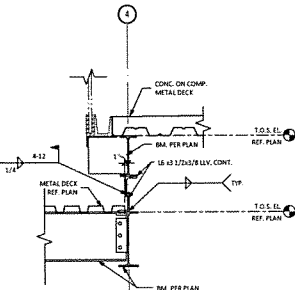
3 DETAIL - MOMENT CONNECTION DETAIL
SCALE: 1" = 1'-0"



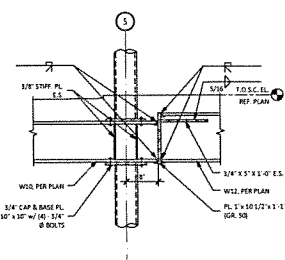
4 WIDE FLANGE TO PIPE COLUMN MOMENT CONNECTION
SCALE: 1" = 1'-0"



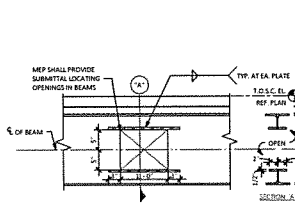
5 DETAIL - ROOF EDGE AT BAR GRATING
SCALE: 1" = 1'-0"



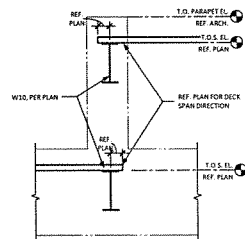
6 DETAIL
SCALE: 1" = 1'-0"



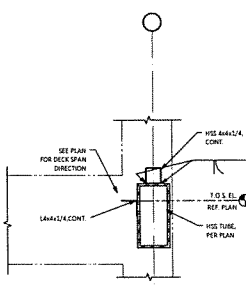
7 DETAIL - MOMENT CONNECTION 2
SCALE: 1" = 1'-0"



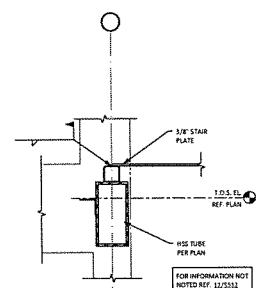
10 DETAIL - REINFORCING AT WEB OPENING
SCALE: 1" = 1'-0"



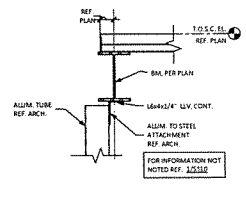
8 DETAIL - SECTION @ ROOF PARAPET
SCALE: 1" = 1'-0"



12 DETAIL - HSS TUBE TO ROOF CONNX.
SCALE: 1" = 1'-0"



13 DETAIL - SPANDREL BEAM @ STAIR PLATE
SCALE: 1" = 1'-0"



14 DETAIL - ALUMINUM TUBE TO WF BEAM CONNX.
SCALE: 1" = 1'-0"

1" = 30'

txva
Architectural
Wolfe
architects

aec
Architectural
Engineers
Consultants

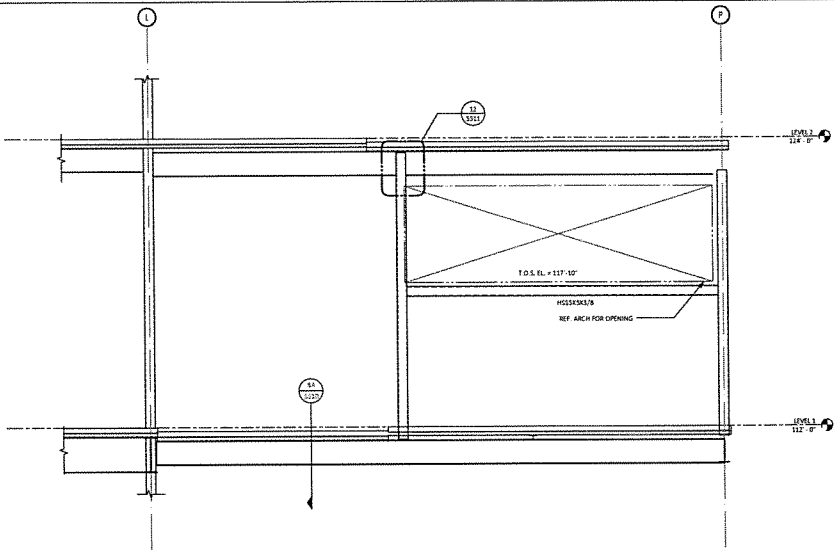
Sheffield Residence
3509 Westlake Drive
Austin, TX

JOB NO. 13067
DRAWING TITLE
STEEL DETAILS
REVISIONS
DATE 04/08/16
DRAWN BY N.K. J.C.
CHECKED BY R.L.
SHEET NO.

S512



Sen-Gin Liu
04.08.2016



1 CB-1 ELEVATION
SCALE: 1/2" = 1'-0"

100 West Texas Avenue
Midland, Texas 79701
432.682.1232
432.682.1237 fax

twa
architects
willen

aec
Engineering
Collaborative
100 West Texas Avenue
Midland, Texas 79701
432.682.1232
432.682.1237 fax

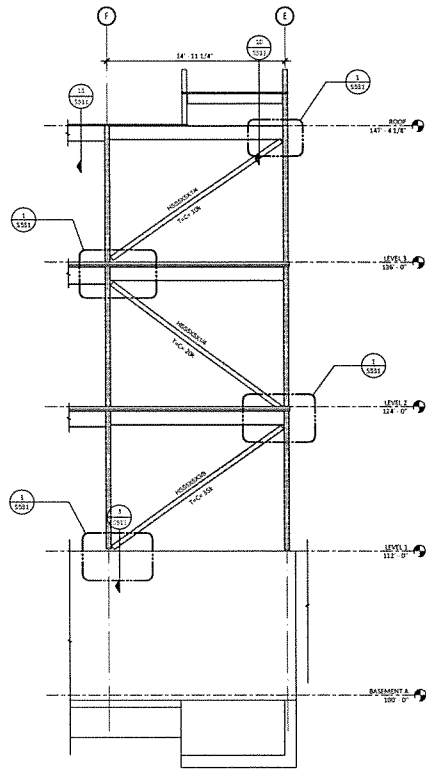
Sheffield Residence
3509 Westlake Drive
Austin, TX

JOB NO. 13057
DRAWING TITLE
CLERESTORY
BRACING ELEVATIONS
REVISIONS
DATE 04/08/16
DRAWN BY N.K. J.C.
CHECKED BY RL
SHEET NO

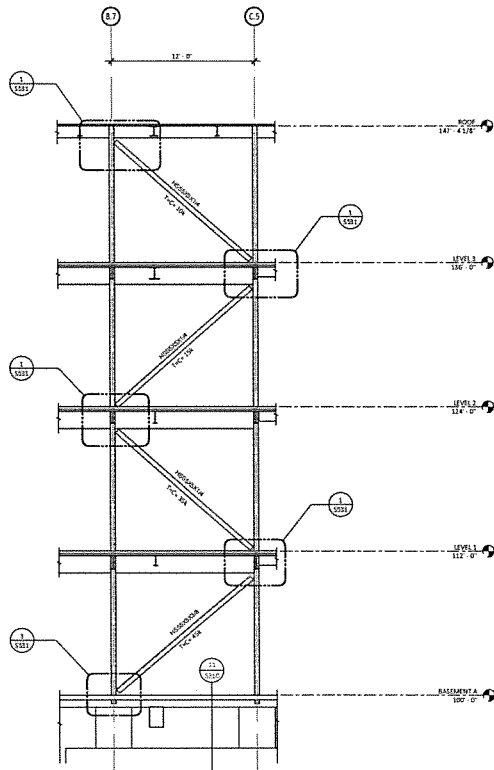
S513



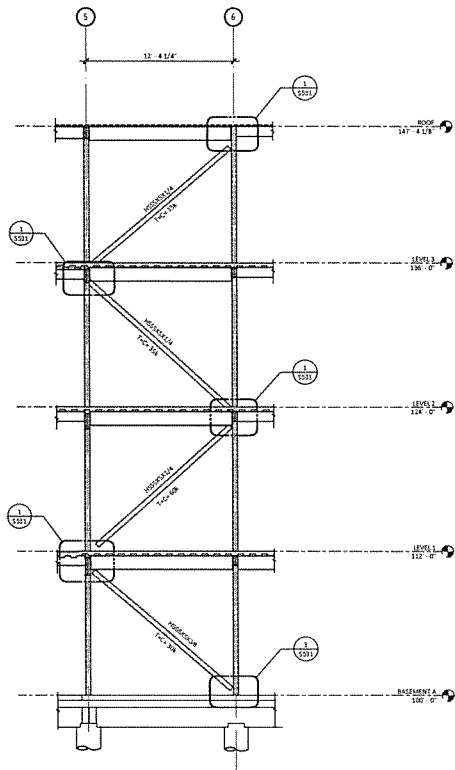
Sen-Gin Liu
04.08.2016



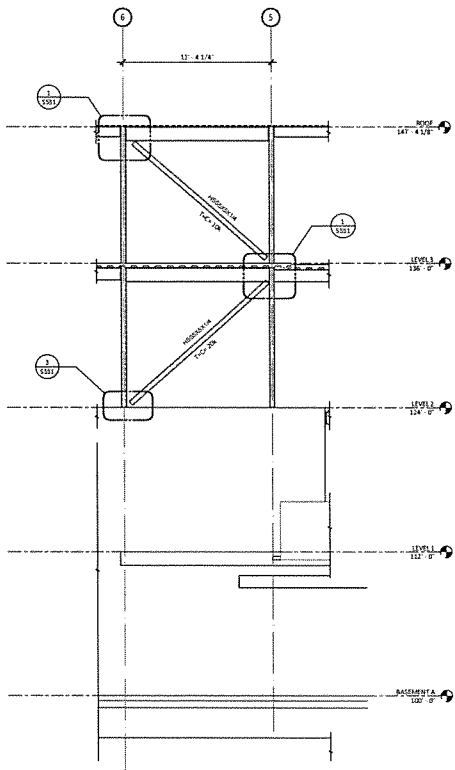
1 ELEVATION - WB-1
SCALE: 1/4" = 1'-0"



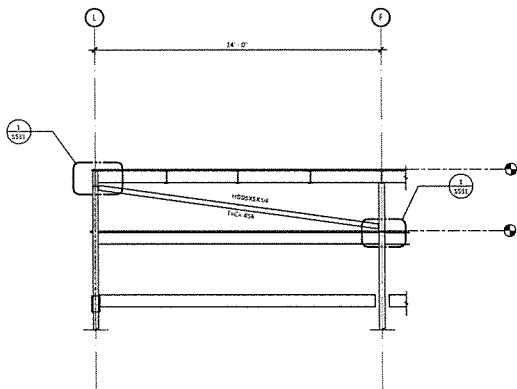
2 ELEVATION - WB-2
SCALE: 1/4" = 1'-0"



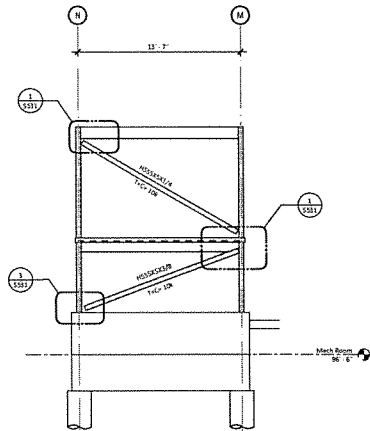
3 ELEVATION - WB-3
SCALE: 1/4" = 1'-0"



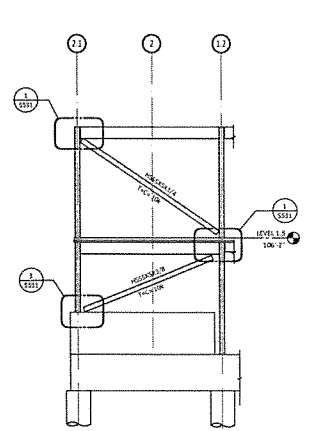
4 ELEVATION - WB-4
SCALE: 1/4" = 1'-0"



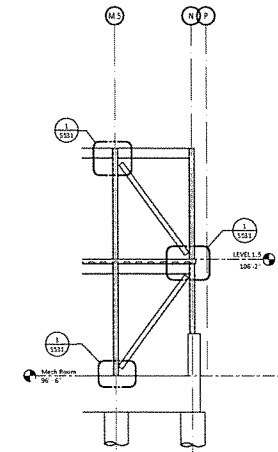
5 ELEVATION - WB-5
SCALE: 1/4" = 1'-0"



7 ELEVATION - WB-7
SCALE: 1/4" = 1'-0"



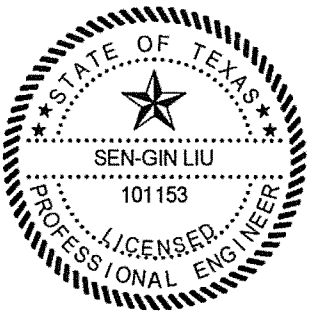
8 ELEVATION - WB-8
SCALE: 1/4" = 1'-0"



6 ELEVATION - WB-9
SCALE: 1/4" = 1'-0"

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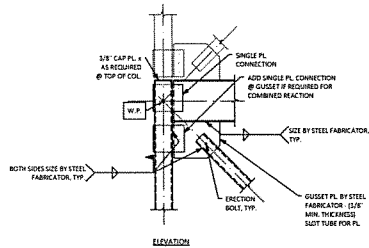


Sen-Gin Liu
04.08.2016

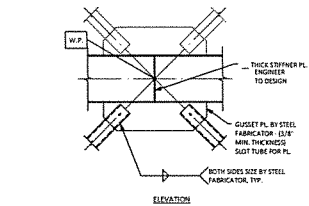
Sheffield Residence
3509 Westlake Drive
Austin, TX

JOB NO. 13067
DRAWING TITLE
WINDOW ELEVATIONS
REVISIONS
DATE 04/08/16
DRAWN BY H.K. J.C.
CHECKED BY R.L.
SHEET NO. S530

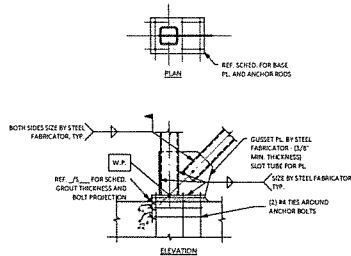
1/16" = 1'



TYPICAL DETAIL
WIDE FLANGE BEAM AND WIND
BRACE CONNECTION TO TUBE COLUMN
NO SCALE



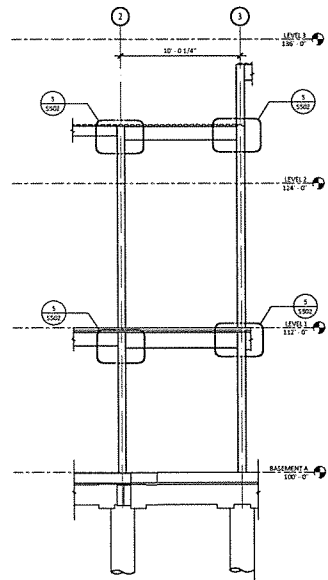
TYPICAL DETAIL
WIND BRACE CONNECTION TO BEAM
NO SCALE



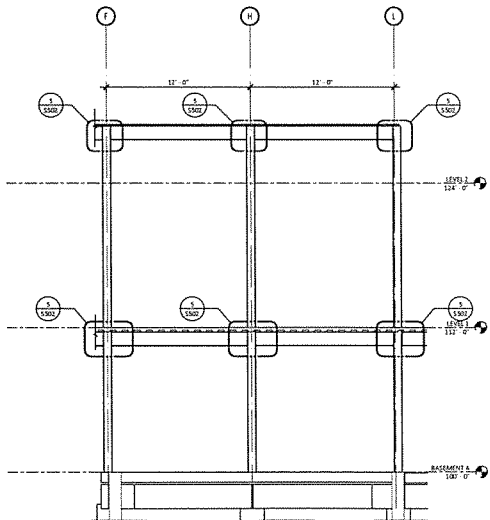
TYPICAL DETAIL
WIND BRACE CONNECTION TO TUBE COLUMN AND BASE PLATE
NO SCALE

WIND BRACE DESIGN NOTES

- Design connections for forces shown on wind brace elevations.
- If no forces are shown on wind brace elevations, design for the full tensile capacity of the brace according to the Manual of American Institute of Steel Construction, Thirteenth Edition.
- See details for forces & eccentricity of C.G. weld & C.G. force.
- At beam to column connections, size attachment to column for combined gravity load shear plus vertical shear component due to lateral load.
- Check for web-toe section buckling and block shear rupture of gusset plates.
- Calculations shall be performed and sealed by a Registered Professional Engineer & shall be submitted to the Architect per specifications.
- Design forces noted as P_{w-2} are factored forces in axial tension or compression. P_{w-3} are factored vertical shear forces.



4 ELEVATION - MOMENT FRAME
SCALE: 1/4\"/>

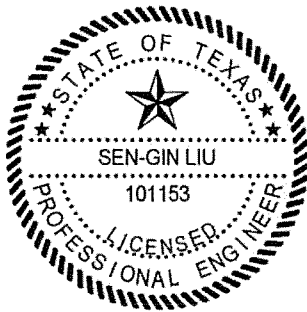


5 MOMENT FRAME - 2
SCALE: 1/4\"/>

1/16" = 1'

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JOB NO. 13067
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WIND BRACE HSS
TYPICAL DETAILS
REVISIONS

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CHECKED BY R.L.
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S531